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22 Abstract

An interlayer soil identified in the French conventional rail track corresponded to a mixture of 23 24 fine soil and coarse grains. To investigate the role of fines in the soil-water retention property of such mixture, different coarse grain contents f_v and dry densities of fine soil ρ_{d-f} were 25 considered. The filter paper method was applied to measure the matric suction. Mercury 26 intrusion porosimetry tests were performed for the microstructure observation of fine soil. In 27 28 terms of gravimetric water content of fine soil w_f with matric suction ψ , the soil-water retention curve (SWRC) was significantly affected by ρ_{d-f} for $\psi < 715$ kPa, while independent 29 of ρ_{d-f} for $\psi > 715$ kPa. Interestingly, this threshold ψ corresponded to a delimiting pore 30 diameter of bi-modal microstructure of fine soil, which separated micro-pores from macro-31 pores. In terms of degree of saturation S_r with ψ , the SWRC was significantly affected by ρ_{d-f} 32 in the full suction range, while independent of f_v . These findings help better understand the 33 results on samples with the dry density of mixture ρ_d kept constant: an increase of f_v resulted 34 in a decrease of ρ_{d-f} and the suction changed accordingly. In that case, both f_v and ψ affected 35 the mechanical behavior. 36

37 Keywords: soil-water retention; matric suction; pore size distribution; dry density; coarse
38 grain content

39 INTRODUCTION

In the French conventional rail track, an interlayer was naturally created in the substructure under the long-term traffic loading, which corresponded to a mixture of ballast grains and subgrade fine soil. As its suction can greatly affect its mechanical behavior (see details in Su et al. 2020a and Wang et al. 2018), it appears important to investigate its water retention property in-depth.

45 In-situ investigations showed a decrease of ballast grain content with the increasing depth of interlayer soil (Trinh 2011, Cui et al. 2013). Globally, the interlayer soil was separated into 46 47 two parts: the upper part was characterized by a coarse grain skeleton fabric, and the lower part by a fine matric macrostructure with dispersed coarse grains. In addition, due to the 48 different vertical stresses over depths, the dry density of fine soil ρ_{d-f} was different. Wang et al. 49 (2017, 2018), Cui (2018) and Qi et al. (2020a) studied the effect of coarse grain content f_v (the 50 ratio of the volume of coarse grains V_c to the volume of total sample V) on the mechanical 51 behavior of interlayer soil at a constant $\rho_{d-f} = 1.82 \text{ Mg/m}^3$. They assumed that under a given 52 water content, the matric suction of mixture was the same at a constant $\rho_{d-f} = 1.82 \text{ Mg/m}^3$. Up 53 to now, this point has not been experimentally examined yet. It seems necessary to verify this 54 point by investigating the effects of coarse grain content f_v and dry density of fine soil ρ_{d-f} on 55 the soil-water retention property of fine/coarse soil mixture. 56

57 The effects of dry density and coarse grain content on soil-water retention property were 58 investigated previously in some studies. The filter paper method (ASTM D5298-10, 2010) 59 was usually adopted for the measurement of suction (Muñoz-Castelblanco et al. 2010; Kim et 60 al. 2015; Jing 2017). It appeared that the effect of dry density on soil-water retention behavior

was strongly dependent on the microstructure (Simms and Yanful 2002; Romero et al. 2011). 61 Romero et al. (1999) studied the effect of dry density on water retention and microstructure of 62 63 Boom clay by vapor equilibrium technique and mercury intrusion porosimetry tests, respectively. The results showed that the soil-water retention curve (SWRC) in the low 64 suction range was governed by the inter-aggregate pores, while that in high suction range was 65 governed by intra-aggregate pores. Similarly, Salager et al. (2013) and Gao and Sun (2017) 66 67 investigated the water retention capacity of clayey soil and found that the SWRCs for different dry densities was independent of dry density beyond a certain matric suction. It is 68 69 worth noting that these studies only involved the effect of dry density without the effect of coarse grain content. Fiès et al. (2002) studied the soil-water retention property of fine 70 71 soil/glass fragments mixture, and found that an increasing glass content led to a reduction of the amount of water stored in the mixture. Baetens et al. (2009) investigated the effect of rock 72 fragments on the water retention property of stony soil, and reported that rock fragments could 73 affect the SWRC when the matric suction was smaller than 30 kPa. Duong et al. (2014) 74 studied the hydraulic behavior of interlayer soil by infiltration column, and observed that 75 increasing coarse grain content resulted in a lower SWRC or a lower water retention capacity. 76 Note that in most of these studies, the effect of coarse grain content on soil-water retention 77 property of mixture was investigated with a large quantity of coarse grains, which 78 corresponded to the coarse grain skeleton structure of mixture, without considering the fine 79 matric macrostructure. In addition, the dry density of mixture ρ_d was taken constant, leading 80 to a decrease of dry density of fine soil ρ_{d-f} with the increase of coarse grain content. That 81 would increase the difficulty of analysis while studying the effect of suction. 82

This study aims to investigate the effects of coarse grain content f_{y} and dry density of fine 83 soil ρ_{d-f} on soil-water retention property of fine/coarse soil mixture. Three $f_v = 0\%$, 20% and 84 35% were adopted at the same $\rho_{d-f} = 1.82 \text{ Mg/m}^3$ for studying the effect of f_v , and three ρ_{d-f} 85 =1.82, 1.67 and 1.52 Mg/m³ were adopted at the same $f_v = 0\%$ for studying the effect of ρ_{d-f} . 86 87 The filter paper method was applied to measure the matric suction of soil mixture. The microstructure of fine soil under varying ρ_{d-f} values was determined by mercury intrusion 88 89 porosimetry tests. The results obtained allowed the effects of f_v and ρ_{d-f} on soil-water retention property of soil mixture to be clarified. 90

91

92 MATERIALS AND METHODS

93 *Fine soil and coarse grains*

Considering the difficulty of extracting intact interlayer soil from the field, the reconstituted 94 fine soil and coarse grains were fabricated in the laboratory. For the fine soil fraction, to 95 obtain a similar grain size distribution of fines from 'Senissiat site' (Trinh 2011) (Fig. 1), nine 96 different mass proportions of commercial soils were mixed (Table 1; see details in Lamas-97 Lopez 2016). The liquid limit and plasticity index of the reconstituted fine soil were 32% and 98 20%, respectively. Consequently, a good agreement between in-situ fine soil and reconstituted 99 fine soil was observed in terms of grain size distribution, plasticity index and liquid limit (see 100 details in Wang et al. 2017). Fig. 2 presents the standard Proctor compaction curve of the 101 reconstituted fine soil, defining an optimum water content $w_{opt-f} = 13.7\%$ and a maximum dry 102 density $\rho_{dmax-f} = 1.82 \text{ Mg/m}^3$. Note that the $\rho_{dmax-f} = 1.82 \text{ Mg/m}^3$ of reconstituted fine soil was 103 consistent with the $\rho_{d-f} = 1.80 \text{ Mg/m}^3$ of in-situ fine soil measured by Lamas-Lopez (2016). 104

105 Based on above features, the reconstituted fine soil was considered as representative of the insitu fine soil. For the coarse grains fraction, the micro-ballast was adopted to replace the real 106 107 ballast by following a parallel gradation method adopted by Wang et al. (2017) and Su et al. (2020a). The validity of this method was verified by Qi et al. (2020b), who performed 108 comparisons between micro-ballast and ballast in terms of mechanical behavior under static 109 and cyclic loadings. Note that the scaled fine/coarse soil mixture was used as representative of 110 111 interlayer soil taken from Senissiat (near Lyon, France, Trinh 2011), which was far from the coastal. Thus, the salt content-related osmotic suction was ignored and only the matric suction 112 was taken into account in this study. 113

Parameter f_v , widely adopted in previous studies (Seif El Dine et al. 2010; Wang et al. 2017, 2018; Su et al. 2020a, 2020b), was considered in this study. Based on the definition of f_v (Eq.(1)), which only quantified the amount of dry coarse grains in the mixture, all voids and water were assumed to be contained in the fine soil (Fig. 3), as in Wang et al. (2018) and Su et al. (2020a). Based on this assumption, the degree of saturation S_r of mixture can be related to the fine soil fraction. :

$$f_{\rm v} = \frac{V_{\rm c}}{V} = \frac{V_{\rm c}}{V_{\rm c} + V_{\rm f}} = \frac{V_{\rm c}}{V_{\rm c} + V_{\rm s-f} + V_{\rm w-f} + V_{\rm a-f}}$$
(1)

where $V_{\rm f}$ is the volume of fine soil; $V_{\rm s-f}$, $V_{\rm w-f}$ and $V_{\rm a-f}$ are the volume of fine grains, water and air in the fine soil.

For the soil mixture at varying f_v , ρ_{d-f} and water content w_f of fine soil, the mass of coarse grains m_{s-c} , fine grains m_{s-f} and water content of fine soil m_{w-f} could be determined as follows:

125
$$m_{\rm s-c} = V_{\rm c} \cdot G_{\rm s-c} \cdot \rho_{\rm w} = f_{\rm v} \cdot V \cdot G_{\rm s-c} \cdot \rho_{\rm w}$$
(2)

126
$$m_{s-f} = \rho_{d-f} \cdot V_f = \rho_{d-f} \cdot V \cdot (1 - f_v)$$
(3)

$$m_{\rm w-f} = w_{\rm f} \cdot m_{\rm s-f} \tag{4}$$

128 where G_{s-c} is the specific gravity of coarse grains; ρ_w is the water unit mass.

129 For the soil mixture at a given ρ_{d-f} , the corresponding void ratio of fine soil e_f could be 130 deduced using Eq. (5):

131
$$e_{\rm f} = \frac{G_{\rm s-f}\rho_{\rm w}}{\rho_{\rm d-f}} - 1 \tag{5}$$

132 where G_{s-f} is the specific gravity of fine soil.

133 Then, the void ratio e_m of soil mixture can be determined:

134
$$e_{\rm m} = \frac{V_{\rm v-f}}{V_{\rm c}+V_{\rm s-f}} = \frac{V_{\rm v-f}}{V-V_{\rm v-f}}$$
(6)

$$V_{v-f} = e_f \cdot V_{s-f} \tag{7}$$

136
$$V_{\rm s-f} = \frac{m_{\rm s-f}}{G_{\rm s-f}\rho_{\rm w}} \tag{8}$$

137 where V_{v-f} is the volume of voids in fine soil; V_{s-f} is the volume of fine grains.

Combining Eqs. (1), (3) and (5)-(8), the void ratio e_m of soil mixture at varying f_v and ρ_{d-f} was obtained:

140 $e_{\rm m} = -1 + \frac{1}{1 - \frac{(G_{\rm s-f}\rho_{\rm W} - \rho_{\rm d-f}) \cdot (1 - f_{\rm V})}{G_{\rm s-f} \rho_{\rm W}}}$ (9)

Fig. 4 shows the variations of e_f and e_m with f_v at a constant $\rho_{d-f} = 1.82 \text{ Mg/m}^3$. The e_f and e_m were obtained by substituting $\rho_{d-f} = 1.82 \text{ Mg/m}^3$ and $G_{s-f} = 2.68$ (Duong et al. 2016) into Eqs. (5) and (9), respectively:

- 144 $e_{\rm f} = 0.47$ (10)
- 145 $e_{\rm m} = -1 + \frac{1}{1 0.32 \cdot (1 f_{\rm v})} \tag{11}$

As mentioned before, all voids and water were assumed to be contained in the fine soil.

147 Thus, the S_r represented both degree of saturation of soil mixture and that of fine soil:

$$S_{\rm r} = \frac{V_{\rm w-f}}{V_{\rm v-f}} \tag{12}$$

The filter paper method (ASTM D5298-10, 2010) was used to measure the matric suction. 150 151 Filter paper measurement was generally performed by putting a piece of filter paper between two soil disks to attain suction equilibrium between filter paper and soil disks (ASTM D5298-152 153 10 2010; Muñoz-Castelblanco et al. 2010; Kim et al. 2015; Jing 2017). Considering the maximum diameter d = 20 mm of coarse grains (Fig. 1), the soil disk was prepared at 100 mm 154 diameter and 100 mm height. As for the preparation of soil disks, the fine soil (Fig. 5(a)) was 155 prepared at a molding water content $w_{opt-f} = 13.7\%$, then stored in a container for at least 24 h, 156 allowing water homogenization. After that, the fine soil was thoroughly mixed with the coarse 157 grains (Fig. 5(a)) with their pre-determined masses at target f_v and ρ_{d-f} values (Table 2). Note 158 that a characteristic coarse grain content $f_{v-cha} \approx 27\%$ identified by Wang et al. (2018) 159 separated the coarse grain skeleton fabric ($f_v > f_{v-cha}$) and the fine matrix macrostructure ($f_v < f_{v-cha}$) 160 f_{v-cha}) of mixture. Both fabrics were considered in this study with f_v ranging from 0% to 35%. 161 The fine/coarse soil mixture were then compacted in four layers, with an equal amount of fine 162 soil and coarse grains for each layer (Fig. 5(b)). Table 2 presents the variations of e_f , S_r and the 163 dry density of soil mixture ρ_d with f_v and ρ_{d-f} at a molding water content of fine $w_{opt-f} = 13.7\%$. 164 After compaction, the soil disks at different f_v and ρ_{d-f} values were wetted from the molding 165 states to the saturated state. The approach proposed by Su et al. (2020c) was adopted during 166 the wetting process: 10 g water was distributed uniformly on the surface of soil disk by a 167 sprayer each time. The disk was then covered with plastic film for at least 7 h. Fig. 2 shows 168 that the wetting process of soil disk from a molding state to a nearly saturated state induced a 169 slight decrease of ρ_{d-f} due to the swelling of fine soil, and consequently a slight increase of e_f 170

and slight decrease of ρ_d (Table 2). The measured w_f and the corresponding S_r of soil disks at nearly saturated state were also presented in Table 2.

To obtain the drying soil-water retention curve of soil mixture at a given f_v and ρ_{d-f} , 10 173 suction measurements were conducted, corresponding to 10 target $w_{\rm f}$ values. When a soil disk 174 175 reached a target $w_{\rm f}$ value, it was covered with plastic film for at least 24 h prior to measuring its matric suction. A set of three filter paper was prepared, with the middle filter paper 176 177 (diameter d = 80mm) slightly smaller than the two outer filter paper (d = 90mm) to avoid contamination of the middle one. The set of three filter papers was then placed between two 178 179 soil disks. The whole set was covered with plastic film, and then sealed with wax. Note that an initial water content of 4.61% of the filter paper was measured, corresponding to a suction 180 181 of 93 MPa. In this case, the filter paper followed a wetting process during the equilibration process between soil and filter paper. After equilibration, the water content of soil disks and 182 the middle filter paper were measured, with a balance of 1/10000 g accuracy. The 183 corresponding matric suction was then determined for varying f_v , ρ_{d-f} and w_f values. It is worth 184 noting that the volume of soil disks at different w_f values was also measured, by means of a 185 caliper. 186

187

188 Mercury intrusion porosimetry test (MIP)

For the MIP tests, three soil disks were prepared at varying $\rho_{d-f} = 1.82$, 1.67 and 1.52 Mg/m³ with the same $f_v = 0\%$. The freeze-drying method was adopted: fine soil was cut into small pieces of around 1.5 g each, and then immersed into nitrogen under vacuum; afterwards, the frozen soil was transferred to the chamber of a freeze dryer for lyophilizing. This method minimized the microstructure disturbance of fine soil, which was widely used in previous
studies (Cui et al. 2002; Delage et al. 2006; Wang et al. 2014).

195

196 RESULTS

197 To determine the equilibration time of samples with the filter paper method, three samples were prepared at the same $f_v = 0\%$, $\rho_{d-f} = 1.82 \text{ Mg/m}^3$ and $w_{opt-f} = 13.7\%$, with 5, 7 and 9 days 198 199 waited, respectively. Fig. 6 depicts the variations of matric suction and water content of filter paper with time. It appears that with the increase of time from 0 to 5, 7 and 9 days, the water 200 content of filter paper increased from 4.61% to 30.59%, 32.02% and 32.11%, and the 201 corresponding matric suction decreased from 93 MPa to 879, 679 and 670 kPa, respectively. 202 203 This indicated that at least a time of 7 days was needed for the suction equilibration between soil and filter paper. Thereby, a duration of 7 days was adopted for all tests. 204

205 Fig. 7 shows the drying SWRC expressed in terms of w_f and S_r with ψ and the variations of $e_{\rm f}$ with ψ for different $f_{\rm v}$ values (0%, 20% and 35%) and different initial $\rho_{\rm d-f}$ values (1.82, 206 1.67 and 1.52 Mg/m³). The retention curves were fitted with the van Genuchten model (1980). 207 The legends were defined following the rule: ' f_v 0%- ρ_{d-f} 1.82' refers to the $f_v = 0$ % and the 208 initial $\rho_{d-f} = 1.82 \text{ Mg/m}^3$. Fig. 7(a) depicts the variations of w_f with ψ for various f_v and ρ_{d-f} 209 values. It appears clearly that the water retention curves were only dependent on the dry 210 density of fines ρ_{d-f} , and independent of the coarse grain content f_v . In addition, the gaps 211 between the three curves for different ρ_{d-f} values decreased with the increase of matric suction 212 ψ . The curves converged to the same one beyond a threshold suction $\psi = 715$ kPa. Thus, the 213 SWRC in low suction range ($\psi < 715$ kPa) was sensitive to the variation of ρ_{d-f} , while 214

215 independent of ρ_{d-f} in high suction range ($\psi > 715$ kPa).

Fig. 7(b) depicts the variations of void ratio of fine soil $e_{\rm f}$ with ψ for the fine/coarse soil mixture at varying $f_{\rm v}$ and $\rho_{\rm d-f}$ values. It appears that such variations were also only dependent on $\rho_{\rm d-f}$ and independent of $f_{\rm v}$. In addition, the lower the $\rho_{\rm d-f}$ value, the larger the decrease of $e_{\rm f}$ with increasing ψ , showing a larger volume change under the effect of suction for the case of lower $\rho_{\rm d-f}$.

Fig.7(c) presents the variations of degree of saturation S_r with ψ for the fine/coarse soil mixture at varying f_v and ρ_{d-f} values. The curves were found to be independent of f_v , which agreed with those in Fig.7 (a)-(b). In addition, the larger the ρ_{d-f} value the higher the water retention capacity. With the decrease of ρ_{d-f} from 1.82 to 1.67 and 1.52 Mg/m³, the air entry value (AEV) decreased from 550 to 96 and 36 kPa respectively.

Fig. 8 shows the pore size distributions (PSD) of fine soil at $\rho_{d-f} = 1.82$, 1.67 and 1.52 226 Mg/m³. It can be observed from Fig. 8(a) that a decrease of ρ_{d-f} resulted in an increase of 227 intruded mercury void ratio $e_{\rm M}$, which was a little smaller than the corresponding global $e_{\rm f}$. 228 Fig. 8(b) presents typical bi-modal porosity of fine soil, with a delimiting diameter d = 0.65229 µm for micro- and macro-pores. For compacted soils, the micro-pores were generally within 230 aggregates (intra-aggregate pores), while the macro-pores were between aggregates (inter-231 aggregate pores) (Delage et al. 1996). With the increase of ρ_{d-f} , the volume of inter-aggregate 232 pores was observed to decrease, while that of intra-aggregate pores was almost constant, 233 suggesting that the compaction process only affected the macro-pores, in agreement with 234 Wang et al. (2014). 235

237 DISCUSSIONS

238 Effect of microstructure of fine soil on SWRC at varying ρ_{d-f}

Fig. 7(a) shows that ρ_{d-f} affected the SWRC only for $\psi < 715$ kPa. Correspondingly, Fig. 8(b) indicates that a decrease of ρ_{d-f} led to an increase of the volume of inter-aggregate pores ($d > 0.65 \mu$ m) without modifying the volume of intra-aggregate pores ($d < 0.65 \mu$ m). The delimiting $d = 0.65 \mu$ m could be associated with an equivalent matric suction based on Laplace's law under the assumption of cylindrical pore shape:

244 $\Psi = \frac{4T_s \cdot \cos \theta}{d} \tag{13}$

where T_s is the surface tension of water, equal to 0.073 N/m at temperature of 20 °C; θ is the contact angle between the liquid-air interface and the solid, taken equal to 0° in this study.

Substituting $d = 0.65 \,\mu\text{m}$ into Eq. (13), the corresponding ψ was obtained:

248 $\psi = 449 \text{ kPa}$ (14)

It was found that this value was close to the AEV of 550 kPa for the mixture at $\rho_{d-f} = 1.82$ 249 Mg/m^3 (Fig. 7(c)), which was consistent with the findings of Zhang et al. (2018). This value 250 was smaller than the threshold $\psi = 715$ kPa in Fig. 7(a). This difference could be explained as 251 follows: the fine soil with varying ρ_{d-f} values for MIP tests was compacted at a molding water 252 content $w_{opt-f} = 13.7\%$, while the fine soil corresponding to the threshold point in Fig. 7(a) was 253 subjected to a saturation process from the molding water content $w_{opt-f} = 13.7\%$, followed by a 254 drying process. Li and Zhang (2009) studied the effect of wetting-drying history on bi-modal 255 porosity of soil, and found that the drying process induced shrinkage of soil, leading to 256 smaller intra-aggregate pores. Similarly, Sun and Cui (2020) investigated the soil-water 257 retention curve of reconstituted silt, and reported that the drying process led to a shrinkage of 258

259 soil and a smaller value of diameter of voids. It could be thus inferred that the larger threshold $\psi = 715$ kPa in Fig. 7(a) was the consequence of pore size decrease due to soil shrinkage 260 shown in Fig. 8(b). It could be thus deduced that in low suction range ($\psi < 715$ kPa) the 261 SWRC was governed by inter-aggregate pores, while in high suction range ($\psi > 715$ kPa) the 262 SWRC was governed by intra-aggregate pores. The similar phenomenon was reported by 263 Salager et al. (2013) while studying the water retention property of clayey soil. They 264 265 identified a threshold suction $\psi = 5000$ kPa separating the inter-aggregate governing suction from intra-aggregate governing suction. With the decrease of ρ_{d-f} , the volume of inter-266 aggregate pores increased (Fig. 8(b)), resulting in an increase of water volume in inter-267 aggregate pores; thereby, an increase of w_f was observed (Fig. 7(a)). However, such decrease 268 of ρ_{d-f} did not affect the volume of intra-aggregate pores (Fig. 8(b)); thus, a constant w_f was 269 observed in high suction range (Fig. 7(a)). This also confirmed that the compaction effort 270 greatly affected the inter-aggregate pores without touching the intra-aggregate pores (Delage 271 et al. 1996). 272

Two categories of fine soil in the mixture at $f_v = 35\%$ were reported by Su et al. (2020c, 273 2021): a relatively dense fine soil in between coarse grains and a relatively loose fine soil in 274 macro-pores among coarse grains. In other words, while compacted to $\rho_{d-f} = 1.82 \text{ Mg/m}^3$, the 275 dense fines had a higher ρ_{d-f} and the loose fines had a lower ρ_{d-f} , with the global ρ_{d-f} being 1.82 276 Mg/m³. Thereby, a higher SWRC was expected for the dense fine soil in between coarse 277 grains and a lower SWRC for the loose fine soil in macro-pores among coarse grains. 278 However, as the SWRC at $f_v = 35\%$ was the same as that at $f_v = 0\%$ and 20% for $\rho_{d-f} = 1.82$ 279 Mg/m³ (Fig. 7(c)), it could be inferred that in spite of the inhomogeneous distribution of fine 280

soil in the mixture, the SWRC appeared to be controlled by the global dry density of fine soil ρ_{d-f} only. Similarly, Zeng et al. (2020) studied the axial swelling property of compacted bentonite/claystone mixture, and found that this behavior was mainly dependent on the global ρ_d of mixture, irrespective of its heterogeneity.

285

286 Comparison of present study at constant ρ_{d-f} with previous study at constant ρ_d

287 In present study, the different $f_v = 0\%$, 20% and 35% corresponded to the same SWRC under the constant $\rho_{d-f} = 1.82 \text{ Mg/m}^3$ (Fig. 7(c)). On the contrary, Duong et al. (2014) investigated 288 the hydraulic behavior of the upper part interlayer soil with two different $f_v = 50.3\%$ and 289 55.5% and a constant dry density of mixture $\rho_d = 2.01 \text{ Mg/m}^3$ by infiltration column (Table 3). 290 Fig. 9 shows that an increase of f_v from 50.3% to 55.5% led to a lower SWRC under the 291 constant $\rho_d = 2.01 \text{ Mg/m}^3$. This phenomenon could be attributed to the effect of ρ_{d-f} on SWRC. 292 Fig. 10 shows a constant $e_m = 0.33$ (corresponding to $\rho_d = 2.01$ Mg/m³) for $f_v = 50.3\%$ and 293 55.5% in Duong et al. (2014), which was different from that in Fig. 4 of present study. While 294 increasing f_v from 50.3% to 55.5%, the e_f was increased from 1.01 to 1.28; thereby, a decrease 295 of ρ_{d-f} from 1.33 to 1.17 Mg/m³ (Table 3). As a result, the increase of f_v from 50.3% to 55.5% 296 led to a lower SWRC (Fig. 9). 297

298

299 The maximum degrees of saturation S_r of samples

300 It appears from Fig. 7(c) and Fig. 9 that the SWRCs all started at a degree of saturation S_r

- 301 lower than 100%. Moreover, the values in Fig. 9 ($S_r = 91\%$ and 78% for $f_v = 50.3\%$ and 55.5%
- 302 respectively) were smaller than the values around $S_r = 95\%$ for varying $f_v = 0\%$, 20% and 35%

303 at $\rho_{d-f} = 1.82 \text{ Mg/m}^3$ in Fig. 7(c), showing that the maximum S_r value reached during the saturation process decreased with the increase of f_v . Saba et al. (2014) worked on a 304 305 compacted sand/bentonite mixture and found that the fine grains of larger sizes (e.g. the maximum d = 2 mm) was preferentially arranged with a large face side in the horizontal 306 307 direction. In addition, due to the restriction of mould wall, more macro-pores were formed in the side part of sample, as evidenced by Zeng et al. (2020) on a compacted 308 309 bentonite/claystone mixture. Such preferential presence of macro-pores in the side part was further confirmed on the mixture of fine soil and micro-ballast by Wang et al. (2018) and Qi et 310 al. (2020a) through observation at X-ray µCT. As these macro-pores could not retain water 311 under the effect of gravity, a maximum value of $S_r = 95\%$ was obtained. With the increase of f_v , 312 313 the addition of coarse grains increased the mould wall restriction effect, generating thus more macro-pores. This led to a decrease of the initial S_r value of the SWRC with f_v . It is worth 314 noting that Duong et al. (2014) adopted the ballast grains of maximum d = 60 mm to prepare 315 the sample of d = 300 mm and h = 600 mm, while in this study the micro-ballast of maximum 316 d = 20 mm (Fig. 1) was adopted to prepare soil disk of d = 100 mm and h = 100 mm. Much 317 larger mould wall restriction effect was thus expected in the case of Duong et al. (2014) due to 318 the larger dimensions of the grains and the sample. Consequently, much lower maximum 319 degrees of saturation ($S_r = 91\%$ and 78% for $f_v = 50.3\%$ and 55.5%) were obtained in their 320 321 case.

322

323 CONCLUSIONS

324 To investigate the role of fine soil on the soil-water retention property of fine/coarse soil

mixture, three $f_v = 0\%$, 20% and 35% at the same $\rho_{d-f} = 1.82 \text{ Mg/m}^3$ and three $\rho_{d-f} = 1.82$, 1.67 and 1.52 Mg/m³ at the same $f_v = 0\%$ were considered. A filter paper method was applied to measure the matric suction of soil mixture at different water contents. Mercury intrusion porosimetry tests were performed for microstructure observation of fine soil at varying ρ_{d-f} . The results obtained allowed the following conclusions to be drawn.

The drying SWRC of mixture was found to be only dependent on ρ_{d-f} and independent of 330 331 $f_{\rm v}$. A typical bi-modal microstructure of fine soil was identified for varying $\rho_{\rm d-f}$ values, defining a micro-pore and a macro-pore populations. When expressed in terms of w_f with ψ , 332 the SWRC was found to be significantly affected by ρ_{d-f} for the matric suction ψ lower than 333 715 kPa. By contrast, when ψ was higher than 715 kPa, the SWRC kept the same, 334 independent of ρ_{d-f} . Interestingly, a delimiting pore diameter $d = 0.65 \ \mu m$ was identified, 335 separating micro-pores (or intra-aggregate pores) from macro-pores (or inter-aggregate pores). 336 This delimiting diameter corresponded to a matric suction of 449 kPa, which was smaller than 337 the threshold $\psi = 715$ kPa due to the effect of volume change experienced in the course of soil 338 wetting/drying from the remolded state ($w_{opt-f} = 13.7\%$). Thus, the SWRC in low suction range 339 ($\psi < 715$ kPa) was governed by macro-pores, while in high suction range ($\psi > 715$ kPa) by 340 micro-pores. In addition, the SWRC of mixture appeared to be controlled by the global fine 341 soil dry density ρ_{d-f} only. The effect of the possible heterogeneity of fine soil distribution 342 inside the mixture seemed to be negligible. 343

When expressed in terms of S_r with ψ , the SWRC appeared to be significantly affected by ρ_{d-f} , while unaffected by f_v . This helped better understand the observation of Duong et al. (2014) – the water retention capacity was decreased by the increase of f_v : in the study of 347 Duong et al. (2014), a constant $\rho_d = 2.01 \text{ Mg/m}^3$ was adopted instead of a constant ρ_{d-f} . In this 348 case, an increase of f_v resulted in a decrease of ρ_{d-f} , thereby a decrease of water retention 349 capacity.

The initial S_r of the SWRC appeared to decrease with the increase of f_v . At $f_v = 0\%$, due to the effect of mould wall restriction, macro-pores were formed in the side part of sample. As these macro-pores could not retain water under the effect of gravity, the maximum value of S_r = 95% smaller than 100% was obtained in the saturation process. With the increase of f_v , the addition of coarse grains increased the mould wall restriction effect, generating more macropores and thus lower maximum S_r . This phenomenon was expected to be more pronounced in the case of larger grains and larger sample dimensions.

From a practical point of view, these findings suggest that when investigating the water-357 retention property of interlayer soil, the scaled coarse grains at small size can be used as a 358 substitute for real ballast grains at large size. With the increasing depth of interlayer soil, the 359 decrease of f_v induced no changes on water-retention capacity of interlayer soil, provided that 360 the ρ_{d-f} of fine soil kept constant. When the water retention property is determined, it can be 361 incorporated in the mechanical models to better describe the variation of permanent strain of 362 unsaturated interlayer soil under the effect of cyclic loadings. In addition, it is worth noting 363 that to some extent these findings can be helpful in evaluating the effects of ρ_{d-f} and f_v on the 364 water retention property of mixtures with varying types of coarse and fine soil. 365

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372 NOTATIONS

е	void ratio
ef	void ratio of fine soil
<i>e</i> _m	void ratio of fine/coarse soil mixture
$f_{ m v}$	coarse grain content
$f_{ m v-cha}$	characteristic coarse grain content
$G_{ ext{s-c}}$	specific gravity of coarse grains
<i>m</i> a-f	mass of air in fine soil
<i>m</i> _{w-f}	mass of water in fine soil
<i>m</i> _{s-f}	mass of fine grains
<i>M</i> s-c	mass of coarse grains
$ ho_{ m d}$	dry density of sample
$ ho_{ ext{d-f}}$	dry density of fine soil
$ ho_{ m dmax-f}$	maximum dry density of fine soil
$ ho_{ m w}$	water unit mass
Sr	degree of saturation
V	volume of fine/coarse soil mixture
V_{f}	volume of fine soil
$V_{ m v-f}$	volume of voids in fine soil
$V_{\rm a-f}$	volume of air in fine soil
$V_{ m w-f}$	volume of water in fine soil
$V_{\text{s-f}}$	volume of fine grains
Vc	volume of coarse grains
Wopt-f	optimum water content of fine soil

${\mathcal W}_{\mathrm{f}}$	water content of fine soil
Ψ	matric suction

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Table 1. Constitution of fine soil

	Soil classification	Commercial Soil	Mass proportion (%)	Range of grain size (mm)
		C4	16.7	0.0009 - 0.50
		C10	20	0.0009 - 0.25
		HN34	3.3	0.063 - 0.50
	Sand	HN31	3.3	0.16 - 0.63
		HN0.4-0.8	6.7	0.25 - 1
		HN0.6-1.6	6.7	0.32 - 2
		HN1-2.5	13.3	0.32 - 3.20
	Clay	Speswhite	23.3	0.0003 - 0.01
		Bentonite	6.7	0.001 - 0.01
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Table 2. As-compacted and saturated states of fine/coarse soil mixture

<u> </u>	As-compacted state				Saturated state					
$f_{\rm v}$	$\rho_{\rm d\text{-}f}$	P	W _{opt-f}	$S_{\rm r}$	$ ho_{ m d}$	$ ho_{ m d-f}$	P	Measured	Measured	$ ho_{ m d}$
(%)	(Mg/m^3)	$c_{\rm f}$	(%)	(%)	(Mg/m^3)	(Mg/m^3)	$c_{\rm f}$	$W_{\rm f}$ (%)	$S_{\rm r}$ (%)	(Mg/m^3)
	1.82	0.47		78	1.82	1.81	0.48	17.2	95	1.81
0	1.67	0.60		61	1.67	1.65	0.62	22.0	95	1.65
	1.52	0.76	13.7	48	1.52	1.50	0.78	27.3	93	1.50
20	1.82	0.47		78	1.99	1.80	0.49	17.3	95	1.97
35	1.82	0.47		78	2.12	1.80	0.48	17.2	95	2.10

Note: f_v represents the volumetric ratio of coarse grains to the fine/coarse soil mixture. ρ_{d-f} , e_f , w_{f} , and w_{opt-f} represent the dry density, void ratio, water content and optimum water content of fine soil, respectively. S_r represents the degree of saturation of fine soil, which is also the degree of saturation of the mixture. ρ_d represents the dry density of soil mixture.

Table 3. Soil properties of Duong et al. (2014)

-	f_{v} (%)	$ ho_{ m d}$ (Mg/m ³)	e _m	$ ho_{ m d-f}$ (Mg/m ³)	e _f
_	50.3	2.01	0.33	1.33	1.01
	55.5			1.17	1.28

509 Note: e_m represents the void ratio of soil mixture.





519 Fig. 1. Grain size distribution curves of fine soil and micro-ballast (after Wang et al. 2017)

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Fig. 7. Drying soil-water retention curves and variations of void ratios of fine soil with matric suction for varying f_v and ρ_{d-f} values: (a) gravimetric water content of fine soil versus matric suction; (b) void ratio of fine soil versus matric suction; (c) degree of saturation versus matric suction



Fig. 8. Pore size distributions of fine soil at $f_v = 0\%$ and different ρ_{d-f} values: (a) cumulative curves; (b) density function curves





687 Fig. 10. Variations of void ratio with f_v at $\rho_d = 2.01$ Mg/m³ in Duong et al. (2014)