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23 Abstract

Ballast/fines mixture is often found in railway substructure and the overall behaviour 24 25 of tracks is strongly dependent on the mechanical behaviour of this mixture. Since directly testing such mixture with large grains is limited with common laboratory 26 equipment, the consideration of a model material at smaller size is advisable. Parallel 27 gradation method is widely used for this purpose. This study assesses the validity of 28 this method in the case of ballast/fines mixture. Large-scale cyclic triaxial tests were 29 carried out on the ballast/fines mixture at six volumetric coarse grain contents. The 30 31 results obtained were analysed, together with those obtained previously from small-scale triaxial tests on a microballast/fines mixture whose microballast grain size 32 distribution was determined by applying the parallel gradation method. The cyclic 33 34 parameters (permanent strain and resilient modulus) were obtained for the two types of mixture. Results show that for all mixtures two distinct soil fabrics can be 35 identified according to the variations of permanent strain and resilient modulus with 36 the volumetric content of coarse grains f_v : a fine-fine contact structure for $f_v \le 20\%$ 37 and a grain-grain contact structure for $f_v \ge 35\%$. In the case of fine-fine contact 38 39 structure, the permanent strains and resilient modulus values of the ballast specimens are consistent with those of the microballast specimens, evidencing the validity of the 40 parallel gradation method. In the case of grain-grain contact structure, the permanent 41 strains and resilient modulus values are found to coincide globally at the two scales, 42 also justifying the validity of the parallel gradation method, the slight differences 43 between the two scales being attributed to the irregular grain sliding and the 44

- 45 distribution of the fines soils.
- 46
- 47 Keywords: interlayer soil; cyclic triaxial tests; permanent deformation; resilient
- 48 modulus; parallel gradation method

49 **1. Introduction**

Ballast/fines mixture is often found in railway substructure [10,17]. For instance, in 50 51 the French conventional railway lines (representing 94% of the total railway network), there is an interlayer which was formed by interpenetration of ballast and subgrade 52 53 soils under the effect of train circulation [10]. During the renewable program for conventional lines, this layer was maintained since the dry density of the interlayer 54 was as high as 2.4 Mg/m³ and thus a favourable mechanical behaviour was expected 55 [7]. According to the in-situ investigation, the soil fabric of the interlayer is found to 56 57 be greatly dependent on the content of ballast grains [37,38]: the ballast grains are in contact with each other for the upper part with high ballast content, but floated in the 58 fines matrix for the lower part with lower ballast content. Since the ballast/fines 59 60 mixture plays an important role in the overall behaviour of rail tracks, it is important to investigate the mechanical response of such mixture under cyclic loading in terms 61 of permanent deformation and resilient modulus. Considering the large size of ballast 62 63 grains, large-scale triaxial apparatus is normally needed for this purpose. However, since the large testing system is complex and costly, scaling the ballast grains down to 64 a model ballast material with smaller size and performing tests using standard 65 apparatus appear quite promoting. 66

Three methods are commonly used for the scaling purpose: the scalping method [50] which consists in discarding the grains larger than a specified size from the prototype materials, the parallel gradation method [24] which consists in creating a model material at smaller size with its grain size distribution parallel to that of the

prototype material, and the replacement method [14] which consists in replacing the 71 grains from a specified size to the maximum size with the grains from this specified 72 73 size to a smaller size. Among them, the parallel gradation method is regarded to be the most appropriate because it can consider a large range of prototype gradation shape; 74 75 thus, it has been adopted mostly [1,16,29,39,42,43,44,47]. The validity of this method under cyclic loadings was assessed by Sevi and Ge [34] based on the response of the 76 prototype and model grains of ballast grains. Their results in terms of resilient 77 modulus, permanent axial strain and permanent volumetric strain were unfortunately 78 79 not conclusive for validating the parallel gradation method. In addition, only pure ballast grains were tested without fines fraction. 80

In this study, the parameters characterizing the cyclic response (permanent 81 82 strain and resilient modulus) of ballast/fines mixtures representing the interlayer soils in the conventional French railway track-beds were determined by conducting 83 large-scale cyclic triaxial tests. Six volumetric ballast contents (0%, 5%, 10%, 20%, 84 85 35%, 45%) were considered. The results obtained were analysed together with those obtained previously by Wang et al. [42,43] from small-scale cyclic triaxial tests on 86 microballast/fines mixture with microballast prepared by applying the parallel 87 gradation method. Comparison of the mechanical parameters obtained at two scales 88 allowed the validity of the parallel gradation method to be assessed on coarse 89 grain/fines mixtures. 90

92 **2. Materials**

The ballast grains having the same grain size distribution as the interlayer soil in 93 "Sénissiat site" were used [37], as shown in Fig. 1. The minimum grain size D_{\min}^{b} 94 and the maximum grain size D_{max}^{b} are 20 mm and 63 mm, respectively. The 95 microballast was tested previously by Wang et al. [42,43]; its maximum grain size 96 D_{\max}^m was chosen as 20 mm to adapt to a small-scale triaxial cell of 100 mm with a 97 value of 5 as the ratio of specimen to maximum grain size. This is consistent with the 98 recommendation of Fagnoul and Bonnechere [12], Nitchiporovitch [25] and Pedro 99 100 [28]: a ratio larger than 5 must be adopted to minimize the specimen size effect. The grain size distribution of the microballast is transited from that of the ballast by 101 following the parallel gradation method. In this method, the correlative grain sizes of 102 103 ballast and microballast share the same percentage of grain passing. The grain sizes of these two materials are correlated according to the following equation: 104

105 (1)
$$\frac{D^b - D^b_{\min}}{D^m - D^m_{\min}} = \frac{D^b_{\max} - D^b_{\min}}{D^m_{\max} - D^m_{\min}} = A$$

where D_{max} , D_{min} and D are the maximum grain size, the minimum grain size, and a given grain size, respectively. The superscripts b and m stand for ballast and microballast, respectively. A is a constant.

To prepare the microballast, three angular-shaped commercial coarse grains were used. Visually, the grain shape of the chosen commercial grains was similar to that of the field ballast from "Sénissiat site". The minimum grain size of the commercial grains D_{\min}^m was 1.6 mm, defining a value of 2.337 for parameter A. For a given grain size of microballast D^m , 10.0 mm for instance, equation (1) gives a 114 correlated ballast grain size D^b of 39.6 mm. As the ballast grain passing percentage 115 is 59.1 % for $D^b = 39.6$ mm, according to the parallel gradation method, the 116 microballast grain passing percentage is also 59.1 % for $D^m = 10$ mm. Following this 117 procedure, the complete grain size distribution of the microballast was obtained, as 118 shown in Fig. 1. This target (parallel gradation) curve was verified by measurement 119 and it is observed that the measured grain size distribution and the target one show a 120 good agreement (Fig. 1).

For the fines soils, since obtaining enough quantity from the field was difficult, 121 122 they were fabricated in the laboratory. Nine commercial soils were mixed to simulate the fines soils from "Sénissiat site", which included five kinds of medium sands, two 123 kinds of fine sands and two kinds of clay soils (kaolinite and bentonite). Table 1 listed 124 125 the properties of these soils. The mass proportion of each constituting soil was calculated according to the grain-size distributions of the fines from "Sénissiat site" 126 (Fig. 2). The calculated results were also shown in Table 1. With the proportion and 127 128 the grain size distribution of each soil, the grain size distribution of the fabricated fines can be calculated, which was plotted in Fig. 2 as well. During the mixing 129 procedure, the materials to be mixed were carefully added into the mixer following 130 the sequence of medium sands, water, fine sands, kaolinite and bentonite. The mass of 131 the added water corresponded to a small water content of 4% of the total mixed soils. 132 The aim of water addition was to prevent the small particles loss during the mixing, 133 occurring when fine sands, kaolinite and bentonite were dry mixed with others. 134 Bentonite was added last since it was the most reactive soil with water. With this 135

sequence, most kaolinite and bentonite particles were expected to be adhered to the 136 surface of sands. Using this procedure, a homogeneous mixture can be obtained [21]. 137 Wang et al. [42,43] also adopted this method when preparing specimens of 138 microballast and fines mixture. After preparation, the fabricated fines soil was 139 compared with the field one in terms of grain size distribution and plasticity. As can 140 be observed from Fig. 2, the grain size distribution of the fabricated fines showed a 141 good agreement with that of the in situ one. In addition, the liquid limit and the 142 plasticity index of the fabricated fines, which were 32% and 20%, respectively, 143 matched the target liquid limit (35.5%) and plasticity index (17.3%) well. This soil is 144 classified as lean clay (CL) according to ASTM D2487-11 [3]. Standard Proctor 145 compaction tests were carried out on the fines material following ASTM D698-12 [4]. 146 147 An optimum water content $w_{opt-f} = 13.70\%$ and a maximum dry density $\rho_{dmax-f} = 1.82$ Mg/m^3 were determined. 148

149

150 **3. Experimental methods**

151 **3.1. Specimen preparation**

In order to quantify the coarse grains in a specimen, a parameter namely volumetric content of coarse grains f_v was adopted, which was defined as the ratio of the total volume of coarse grains to the total specimen volume [28,29,32,42,43,44]. The total specimen volume consisted of two parts: the coarse grain volume and the fines volume. All pores were assumed to be included in the fines soils. For the upper part of the natural interlayer soils at "Sénissiat site", the f_v value was 52.9% [37]. In this study, six f_{ν} values (0%, 5%, 10%, 20%, 35%, 45%) were considered for the specimens prepared with both ballast and microballast. For convenience, in further analysis, the specimens prepared with ballast and microballast are referred to as ballast specimens and microballast specimens, respectively. The dimensions of the ballast specimens are 300 mm diameter and 600 mm height, while the dimensions of microballast specimens are 100 mm diameter and 200 mm height.

For the preparation of the ballast specimen, the total volume was first calculated 164 with the specimen dimensions (300 mm diameter and 600 mm height). For a given f_{ν} 165 166 value, the volumes of both ballast grains and fines can be obtained based on this total specimen volume. After that, the mass of ballast was obtained using the dry unit mass 167 of ballast (2.68 Mg/m³). The fines in all specimens were kept at the same state defined 168 169 by the optimum water content $w_{opt-f} = 13.70\%$ and the maximum dry density $\rho_{dmax-f} =$ 1.82 Mg/m³. The optimum state was selected aiming to simulate the heavily compact 170 state in the field condition [8,21,38]. Accordingly, the masses of dry fines and water 171 can be determined. Water was firstly added into the fines by spray to reach the 172 optimum water content $w_{opt-f} = 13.7\%$. Then, the wetted fines soil was stored in 173 hermetic containers for 24 h for moisture homogenization. Afterwards, the 174 homogenized wetted fines soil was equally divided into eight parts (by weight) and so 175 were the oven-dried ballast grains, after which they were mixed separately. Finally, 176 the ballast specimen was compacted using a layered compaction method, which was 177 widely adopted by many researchers to ensure reasonable uniformity of the specimen 178 [5,22,35,46]. The mixed materials were compacted in eight layers using a vibration 179

180 hammer, each with a thickness of 75 mm.

Similar protocol was applied for the preparation of the microballast specimen 181 182 (100 mm diameter and 200 mm height). The masses of the dry microballast, dry fines soil and water were firstly determined. Afterwards, the fines soil was wetted with the 183 sprayed water to reach the optimum water content of 13.7%, stored in hermetic 184 container for 24 h and then mixed with the microballast. Finally, this mixture was 185 compacted dynamically to reach the target height, also following the layered 186 compaction method to ensure reasonable uniformity of the specimen. More details can 187 188 be found in Wang et al. [42,43]. Note that the test results at five f_v values (5%, 10%, 20%, 35%, 45%) obtained by Wang et al. [42,43] were taken for comparison with the 189 190 corresponding ones of the ballast samples in this study.

191

192 **3.2.** Cyclic triaxial tests

For the microballast specimen with 100 mm diameter and 200 mm height, a 193 small-scale cyclic triaxial apparatus was used. Details about this device can be found 194 in Wang et al. [43]. For testing a ballast specimen (300 mm diameter and 600 mm 195 height), a large-scale cyclic triaxial apparatus developed by Dupla et al. [11] was 196 adopted. The loading procedures adopted are the same for the ballast and the 197 microballast specimens. No saturation process was applied and the specimen water 198 content was kept constant during the test. In addition, the drainage valve was kept 199 open during the loading process. All the tests were performed at a confining pressure 200 of 30 kPa, which corresponded to the average horizontal stress estimated for the field 201

condition, considering the effects of the train wheel load, the interlayer depth and the 202 Poisson ratio [9,42,43]. The applied cyclic loads followed a sine-shaped pattern (Fig. 203 204 3). The deviator stress amplitude Δq is defined as the difference value between the maximum deviator stress q_{max} and the minimum deviator stress q_{min} [40,41,42,43]. A 205 frequency of 1.78 Hz corresponding to a low train speed of 50 km/h was chosen, 206 aiming to guarantee the quality of controlling the pre-defined loading shape. Note that 207 this low frequency did not reflect the high train speed normally encountered in the 208 field. This selection was based on the hypothesis that the train speed does not 209 210 influence the assessment of the validity of the parallel gradation. During the loading procedure, axial stress and strain were monitored. 211

The application of cyclic loads included two consecutive stages. The loading 212 213 stage 1 was focused on the aspect of the permanent deformation under a large number of loading cycles. The loading stage 2 was focused on the aspect of the resilient 214 modulus or stiffness property under a wide range of stress amplitudes, including some 215 large amplitudes possibly applied on the interlayer soils. In stage 1, a multi-step 216 loading procedure proposed by Gidel et al. [13] was adopted (Fig. 4a): the deviator 217 stress amplitude $\Delta q = q_{max} - q_{min}$ was increased stepwise from 10 kPa to 30 kPa with 218 an increment of 5 kPa. At each amplitude, 90,000 cycles were applied, which were 219 thought to be enough for the stabilization of the permanent strain [10,13,21,38]. This 220 multi-stage loading procedure allowed the application of several stress amplitudes on 221 the same specimen, with which not only the number of tests could be reduced, but 222 also the effect of the variability of soil specimens on the testing results can be 223

224	minimised. Thus, this procedure has been widely used in the study of the permanent
225	deformation behaviour [6,10,19,21,36,38,43]. These loading amplitudes were selected
226	based on the vertical stress measured at the equivalent depth of interlayer in the field
227	[20,21]. In stage 2 (Fig. 4b), a procedure proposed by Lamas-Lopez [23] was adopted.
228	The deviator stress amplitude Δq was firstly increased following a Δq sequence (10
229	kPa, 30 kPa and 50 kPa) and then decreased following the reverse sequence.
230	Afterwards, Δq was increased following a sequence from 10 kPa to 100 kPa and then
231	decreased to 10 kPa. Finally, Δq was increased in steps to 200 kPa and then decreased
232	to 10 kPa. At each stress level, 100 cycles were applied. The aim of firstly increasing
233	Δq and then decreasing it was to obtain the resilient modulus at both elastoplastic
234	stage and pure elastic stage. Note that the large stress amplitudes (50 kPa, 100 kPa
235	and 200 kPa) were defined according to the vertical stress of 40-90 kPa at similar
236	depth of the interlayer observed by Selig and Waters [33], Jain and Keshav [18] and
237	Yang et al. [49], the vertical stress of 120-138 kPa applied by heavier wagons in some
238	countries [15,18,23] and the contingent maximum vertical stress at the top of railway
239	structure as high as 200 kPa [21].

240

241 **4. Results and discussions**

242 **4.1.** Variations of permanent deformation with f_v

Fig. 5 plots the deviator stress q versus the axial strain ε_1 during the first 90,000 cycles of loading stage 1 for the ballast specimen at $f_v = 35\%$. In a loading-unloading cycle, the permanent axial strain ε_{1p} represented the irreversible part of the total 246 axial strain. It can be observed that the ε_{1p} value of the first cycle was particularly large. This phenomenon was also observed for other ballast specimens and the 247 microballast specimens. This large permanent strain can be attributed to the 248 adaptation of the specimen to the loading system, in particular in terms of contact 249 between the specimen and the loading piston. As the initial specimen-piston contact 250 251 was not identical for all the specimens, this ε_{1p} value would make impossible the relevant comparison of permanent strain between different specimens. Thus, in further 252 analysis, the first cycle was not accounted for the cumulative permanent strain. 253

Fig. 6 presents the permanent strain ε_{1p} against the number of cycles N for six 254 ballast specimens, with the corresponding stress levels. The evolutions of permanent 255 strain with number of cycles N were the same as the ones of microballast specimens 256 257 [43]. It can be observed from Fig. 6 that for each f_v value, ε_{1p} increased quickly at the start of each loading level and then tended to stabilize with the increasing cycles. 258 According to Werkmeister et al. [45], the large deformation in the beginning resulted 259 260 from a significant rearrangement of the particles. As the cycle number increased, this 261 particle rearrangement became more and more limited, and correspondingly, the permanent strain tended to become stable. As can also be seen from Fig. 6, a 262 smaller f_v value led to a larger ε_{1p} , which can be attributed to the involvement of less 263 264 coarse grains.

Due to the multi-step loading procedure adopted, the ε_{1p} at each stress level was greatly influenced by the previous loadings (Fig. 6). If this loading history is eliminated (as if the specimen has been subjected to only one loading level), the effect of the ballast contents can be better evaluated. For this purpose, a permanent strain estimation method proposed by Gidel et al. [13] and then used by Lamas-Lopez [21] and Wang et al. [43] was adopted. The application of this method is explained in Fig. 7 by considering two successive loading levels (Loading level *M* and Loading level *M* + 1). For the estimated permanent strain ε_{1p}^{M+1} of the Loading level *M* + 1 at the first 90,000 cycles, it is determined as:

274 (2)
$$\varepsilon_{1p}^{M+1} = \varepsilon_{1p}^{M} + \delta \varepsilon_{1p}^{M+1}$$

where ε_{1p}^{M} is the measured permanent strain of the Loading Level *M*; $\delta \varepsilon_{1p}^{M+1}$ is the translated permanent strain determined by resetting both the initial strain value and the first cycle of the measured permanent strain of the Loading level *M* + 1 to 0.

For estimating the permanent strain of the Loading level M + 1 after 90,000 cycles, a linear increasing trend between permanent strain and number of cycles is considered, with its slope kept at the same value as the one determined at the last cycles of Loading level M + 1. More details can be found in Wang et al. [43].

The estimated permanent strain curves for $f_v = 0\%$ of the ballast specimen are 282 presented in Fig. 8, together with the measured results. It can be observed that at the 283 last cycle of each stress level, the estimated strain was larger than the measured one. 284 For example, for $\Delta q = 20$ kPa, the measured permanent strain at the last cycle N = 285 270,000 was 0.182%, while the corresponding estimated one was 0.226%. This 286 phenomenon can be attributed to the different loading histories: for the estimated 287 strain, the specimen was assumed to undergo one higher stress level of 20 kPa. When 288 it came to the measured strain, successive lower stress levels of 10kPa, 15 kPa and 20 289

kPa were applied, leading to a smaller strain value.

Gidel et al. [13] examined this permanent strain estimation method through 291 292 cyclic triaxial tests, concluding that the permanent strain estimated for a single stress level agreed well with the measured one for the loading cycles applied at each step 293 (90,000 cycles in this study). In other words, in this study, the estimated permanent 294 strain at the first 90,000 cycles was relatively accurate. Fig. 9 depicts the estimated 295 permanent strain evolutions at the first 90,000 cycles under different stress levels for 296 ballast specimen at $f_v = 0\%$. As can be observed, a larger stress level led to a larger 297 298 strain value. In addition, the permanent strain gradually stabilized with the increase of the cycle number. To further illustrate the effect of ballast content f_{ν} , the estimated 299 permanent strains for cycle N = 90,000 (end-stage cycle in Fig. 9) at all stress levels 300 301 were taken and compared (Fig. 10). It can be observed that at each stress level, the permanent strain decreased with the increase of f_{ν} . Moreover, a bi-linear decreasing 302 trend could be identified. When $f_v \leq 20\%$, the permanent strain decreased rapidly. By 303 304 contrast, when $f_v \ge 35\%$, this decreasing rate became smaller. The same observations were made by Wang et al. [43] for the microballast specimens. 305

In the study of Wang et al. [43], X-ray microcomputed tomography (μ CT) scans were carried out on the as-compacted microballast specimens. Two soil fabrics were identified at different f_v values: a fine-fine contact structure at $f_v = 0\%$ -20% and a distinct grain-grain contact structure at $f_v = 35\%$ -45%. It is worth noting that at $f_v =$ 20%, the coarse grains started to be partially connected without forming a whole skeleton. Unfortunately, this μ CT scan could not be conducted on ballast specimens due to their large size. However, basically, it seems plausible to consider the same feature of fabrics for the ballast specimens: also a fine-fine contact structure at low f_v and a grain-grain contact structure at high f_v . For fine-fine contact structure, the permanent strain results from the compression of the fines, which decreases rapidly with increasing f_v . By contrast, for grain-grain contact structure, the rearrangement of the grains plays a dominant role and the permanent strain decreases slightly when f_v increases.

319

320 **4.2.** Variations of resilient modulus with f_v

Typical hysteresis loops at the first cycles of different stress levels are shown in Fig. 321 11 for the ballast specimen at $f_v = 20\%$. The loops for stress levels of $\Delta q = 10$ kPa, Δq 322 323 = 30 kPa, Δq = 50 kPa, Δq = 100 kPa and Δq = 200 kPa corresponded to cycle numbers of N = 1, N = 101, N = 201, N = 701 and N = 1401, respectively. It appears 324 that for the low stress levels of 10 kPa and 30 kPa, the hysteresis loops were pretty 325 small and closed. This was due to the application of 450,000 loading cycles under Δq 326 \leq 30 kPa in loading stage 1. By contrast, for stress levels larger than 30 kPa, 327 permanent strain developed, leading to larger and unclosed hysteresis loops, in 328 particular under stress levels as high as $\Delta q = 100$ kPa and 200 kPa. 329

The evolution of hysteresis loop with number of cycles is illustrated in Fig. 12 by plotting the loops for 100 cycles (from N = 701 to N = 800) at Δq = 100 kPa for the ballast specimen with f_v = 20%. A large and unclosed hysteresis loop was observed for the first cycle, due to the stress level of 100 kPa which was larger than all the previous applied stress levels. In addition, with the increase of the cycle number, the hysteresis loop became smaller and smaller and at the end of loading, the loops became closed and relatively unchanged, suggesting a pure elastic behaviour of soil.

As shown in Fig. 13, the resilient modulus M_r is defined as [30,31]:

337

338

$$(3) M_r = \Delta q / \varepsilon_1$$

where Δq is the deviator stress amplitude, and ε_{1r} is the resilient strain.

The evolution of M_r with the number of cycles N at varying stress levels is 340 presented in Fig. 14 for the ballast specimen at $f_v = 20\%$. The results of other ballast 341 342 specimens and microballast specimens obey the same rule and are not shown for clarity. It can be observed that when the stress amplitude increased, the M_r value 343 immediately decreased and then tended to stabilize. By contrast, with the stress 344 345 amplitude decreasing, the M_r value increased instantly and then tended to reach a stabilization state. The decrease of M_r with the increase of stress amplitude can be 346 explained by the strain increasing due to the increase of stress amplitude [2]. 347

348 In order to illustrate the effect of f_v on resilient modulus, the M_r values of the last (end-stage) cycles of all stress levels were taken and presented as a function of f_{ν} 349 for different stress levels (Fig. 15). The data were named according to the number of 350 cycles N and the corresponding Δq values. It appears that an increasing trend of M_r 351 with f_v can be identified for all stress levels. Moreover, this increasing trend followed 352 a bi-linear pattern with the slope at $f_v \ge 35\%$ larger than the one at $f_v \le 20\%$, 353 evidencing that the soil fabrics of the ballast specimens at these two f_v ranges were 354 different: fine-fine contact structure at low f_{ν} and grain-grain contact structure at 355

higher f_{ν} . Same bi-linear variation trend of M_r was identified by Wang et al. [42] for the microballast specimens, similarly, evidencing that the soil fabrics of microballast specimens at $f_{\nu} \le 20\%$ and $f_{\nu} \ge 35\%$ were separately dominated by fine-fine contact structure and grain-grain contact structure.

360

4.3. Comparison of permanent strain between ballast and microballast specimens

The estimated permanent strains for cycle N = 90,000 (end-stage cycle) at six f_v values 363 are presented in Fig. 16 for the ballast and the microballast specimens, for five stress 364 levels (10, 15, 20, 25 and 30 kPa). It can be observed that in the case of $f_v = 0\%$, at 365 each stress level, the ε_{1p} value of the ballast specimen coincides well with that of the 366 microballast specimen. For the fine-fine contact structure at $f_v = 5\%$ and 10%, on the 367 whole, the ε_{1p} values of the corresponding different scale specimens are found to be 368 similar. This phenomenon can be explained by the fact that in this case the fines 369 matrix constitutes the soil skeleton and the permanent strain was dominated by the 370 compression of the fines, giving rise to the permanent strains which were independent 371 of coarse grain size. For the grain-grain contact structure at $f_v = 35\%$ and 45%, it can 372 be observed that the permanent strains of the ballast specimens and the microballast 373 specimens were almost the same (Fig. 16), in particular at $\Delta q = 10$ kPa (Fig. 16a). 374 Note that although the case at $f_v = 20\%$ was categorized into the group of fine-fine 375 contact structure, the influence of coarse grain size on the permanent strain followed 376 the same pattern. This can be explained by the fact that at $f_v = 20\%$, part of the coarse 377

grains started to be in contact with each other. Further examination shows that at Δq 378 higher than 10 kPa, the strains of microballast specimens were slightly larger than 379 380 those of the ballast specimens. This phenomenon can be explained as follows: upon loadings, the grains would slide into the pores nearby, giving rise to permanent strain. 381 If the difficulty of sliding into large pores for the large grains was the same as that for 382 the smaller grains to slide into smaller pores, identical permanent strains can be 383 expected. It is the case for the ballast specimen and the microballast specimen at $\Delta q =$ 384 10 kPa. Apparently, the grains sliding depended on the stress level and the grain sizes; 385 386 in the microballast specimen, the grains with smaller sizes would slide into the adjacent pores more easily, leading to a slightly larger strain. 387

Another possible explanation for the slightly larger strains in the microballast 388 389 specimen is related to the effect of the fines soils. As shown in Fig. 17, the fines soils can be roughly considered to be distributed at two locations: the pores near the coarse 390 grain and the coarse grain intervals along the stress chain (supposed to be along the 391 specimen height). The applied load was transmitted along the stress chain by 392 compressing the fines between grains. When the load reached a certain level, grain 393 rearrangement occurred [26,27]. It can be inferred that the fines in the pores inhibited 394 the coarse grain sliding, while those between grains promoted it. At a given f_v value, 395 for the microballast specimen, more grain-fines-grain contacts can be expected along 396 the stress chain, leading to more distribution of fines soils. As a result, relatively 397 larger permanent strain can be expected. 398

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As the difference of permanent strain between the two scale specimens appears

quite small (Figure. 16), it can be reasonably concluded that the parallel gradationmethod is valid for the ballast/fines mixture in terms of permanent strain.

402

403 **4.4. Comparison of resilient modulus between ballast and microballast specimens** 404 The comparison of M_r values between the microballast and ballast specimens with 405 fine-fine contact structure is shown in Fig. 18. At $f_v = 0\%$, the M_r values of the ballast 406 specimen were comparable with those of the microballast specimen. This was normal 407 because the scaling question wasnot involved in that case. For higher f_v values, except 408 $f_v = 10\%$, the M_r values of the ballast and the microballast specimens coincided with 409 each other. The singular case at $f_v = 10\%$ was probably due to a technical problem.

The M_r values of the ballast specimens at grain-grain contact structure were 410 compared with those of the mciroballast specimens in Fig. 19 (Fig. 19a for $f_v = 35\%$ 411 and Fig. 19b for $f_v = 45\%$). It can be observed that almost the same M_r values were 412 obtained for the two specimens, indicating that the resilient modulus was independent 413 of grain size. Further investigation shows that on the whole the M_r values of the 414 microballast specimens were slightly smaller than those of the ballast specimens. 415 These phenomena can be explained based on the analysis of Yang and Gu [48]. They 416 derived the resilient modulus of a simple cubic array of identical spheres under a 417 certain confining pressure by applying a small shear stress onto it. In the analysis, the 418 induced resilient shear strain of the cubic array was determined by adding up the 419 tangential strains between every two spheres along the vertical stress chain. This 420 tangential strain was calculated using the Hertz-Mindlin contact law, which was a 421

function of the applied shear stress. The expression for the resilient modulus (defined as the ratio of the shear stress to the shear strain) was obtained, showing no dependence on sphere radius. Note that this analysis did not consider any contribution from fine soils, thus not fully reflecting the case of this study. Indeed, as mentioned previously, in the case of mixture, the resilient modulus is affected by the fine soils along the stress chain. For the microballast specimens, as stated before, there are more fines soils along it. Thus, relatively smaller resilient modulus values are expected.

As for the permanent strain, globally, the resilient modulus also showed a good agreement between the two scale specimens albeit the slight difference discussed above, which validates the parallel gradation method in terms of resilient modulus.

432

433 **5.** Conclusions

The parallel gradation method was assessed on coarse grain/fines mixture by comparing the mechanical parameters (permanent strain and resilient modulus) of the two different scale specimens. Six volumetric coarse grain contents were considered for each kind of specimens. Based on the results obtained, the following conclusions can be drawn.

Two soil fabrics could be defined as fine-fine contact structure at $f_v = 0.20\%$ and grain-grain contact structure at $f_v = 35.45\%$. It was observed that for the ballast and microballast specimens, with the increase of f_v value, the permanent strain decreased rapidly when $f_v \le 20\%$ and this decreasing rate slowed down when $f_v \ge 35\%$, while the resilient modulus increased slightly when $f_v \le 20\%$ and increased 444 significantly when $f_v \ge 35\%$.

445	The consistency between the large-scale and small-scale triaxial tests were
446	found to be satisfactory: at $f_v = 0\%$, the permanent strain as well as the resilient
447	modulus values obtained from the two scales showed a good correspondence. In the
448	case of fine-fine contact structure, the parallel gradation method was found to be valid.
449	The permanent strain and resilient modulus values of the ballast specimens were
450	consistent with the respective ones of the microballast specimens. This could be
451	explained by the fact that in this case the fines matrix governed the soil structure.
452	In the case of grain-grain contact structure, on the whole, the permanent strains
453	of the ballast specimens coincided with those of the microballast specimens
454	(especially at $\Delta q = 10$ kPa). This coincidence was also observed in terms of resilient
455	modulus. For the permanent strain, it could be explained by the fact that the
456	difficulties of the sliding of large ballast grains into the adjacent large pores and the
457	sliding of the small microballast grains into the smaller pores were expected to be the
458	same. For the resilient modulus, it could be explained by the analytical results of Yang
459	and Gu (2013), showing that the resilient modulus is independent of grain size.
460	Further examination showed that there were slight differences of permanent strain and
461	resilient modulus between two different scale specimens, which could be attributed to
462	the irregular grain sliding and the fines distribution.

463

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467 Notations

A	constant in the similitude equation from ballast to microballast
D^b	given grain size of ballast
D_{\max}^b	maximum grain size of ballast
D^b_{\min}	minimum grain size of ballast
D^m	given grain size of microballast
D_{\max}^m	maximum grain size of microballast
D_{\min}^m	minimum grain size of microballast
f_v	volumetric content of coarse grains
M_r	resilient modulus
Ν	number of cycles
q	deviator stress
q_{max}	maximum deviator stress
q_{min}	minimum deviator stress

Wopt-f	optimum water content of fines
Δq	deviator stress amplitude
$\delta arepsilon_{1p}^{M+1}$	translated permanent strain from the measured curve of Loading level
	M+1
ε_1	axial strain

ε_{1p}	permanent axial strain
ε_{1r}	resilient strain
ε^M_{1p}	measured permanent strain of the Loading Level M
ε_{1p}^{M+1}	estimated permanent strain of Loading level M+1
$ ho_{ m dmax-f}$	maximum dry density of fines

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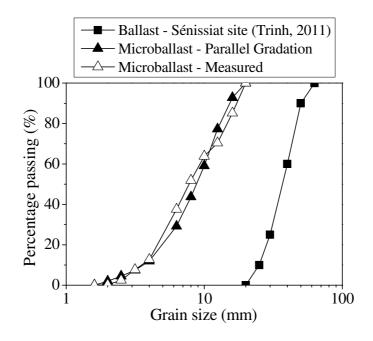


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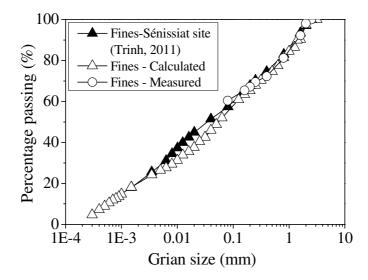


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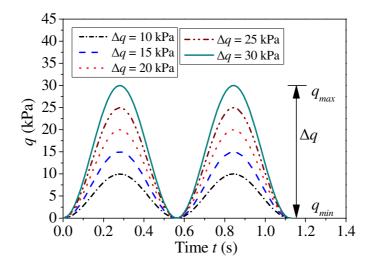


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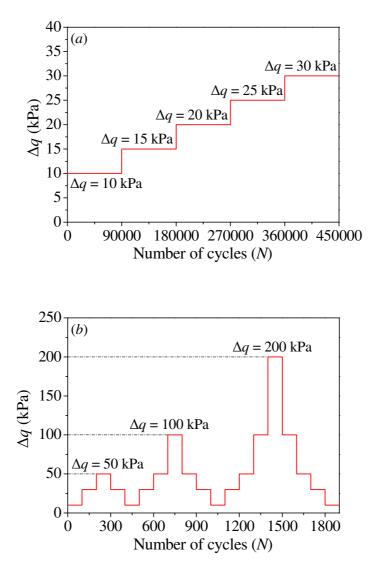


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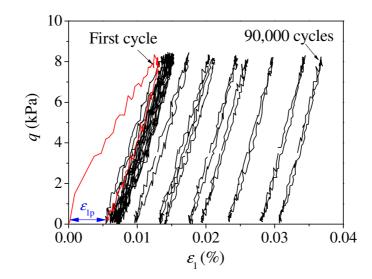


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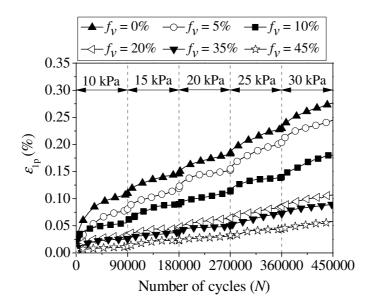


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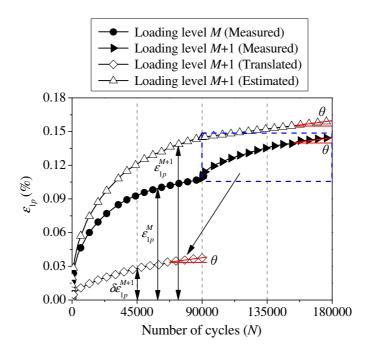


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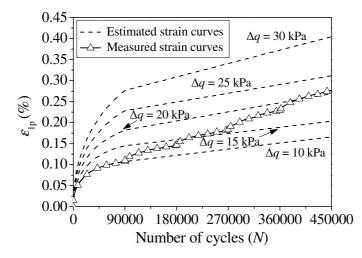


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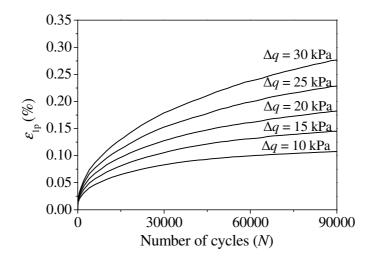


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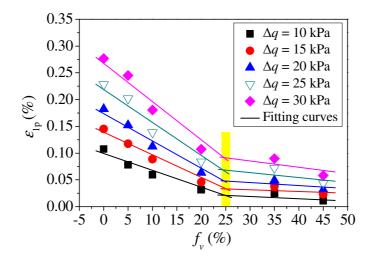


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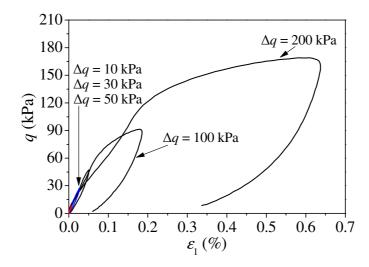


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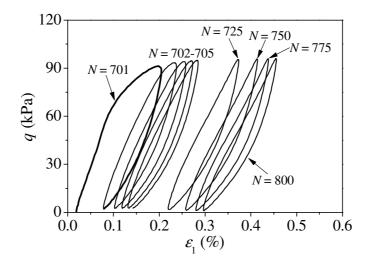


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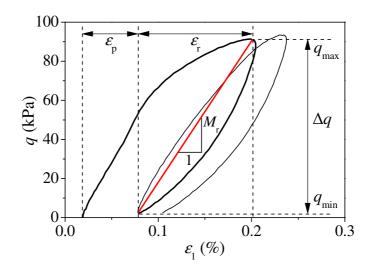


Fig. 13. Determination of M_r

Note: The two hysteresis loops shown are the testing results of ballast specimen at f_v =

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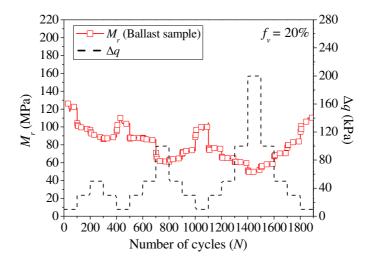


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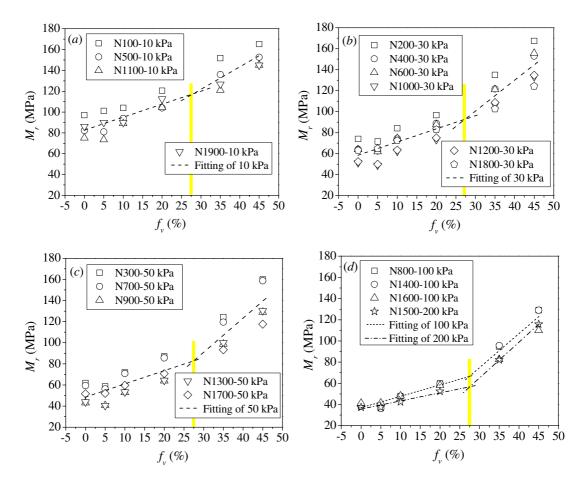


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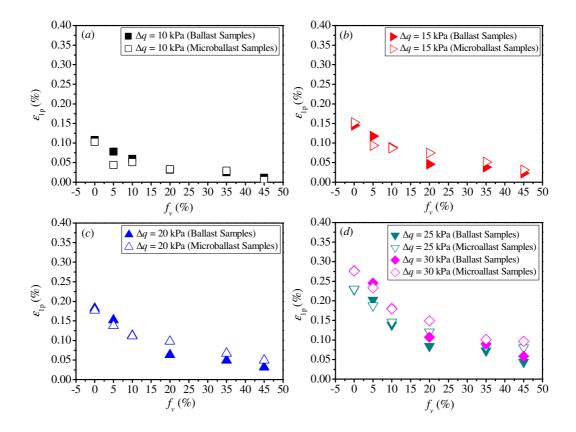


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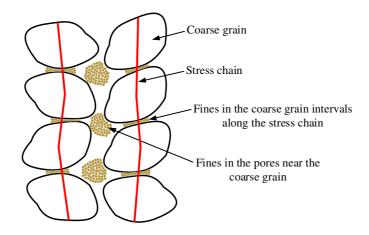


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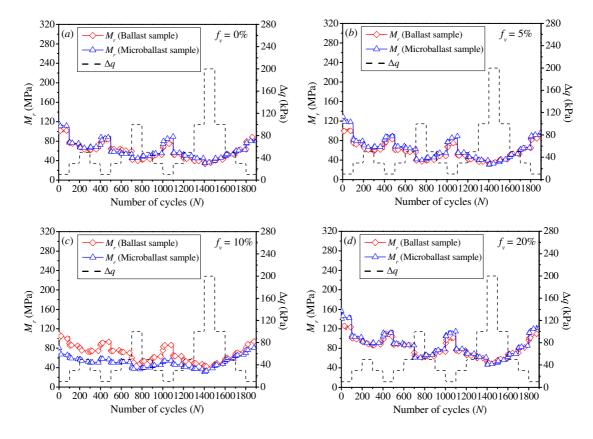


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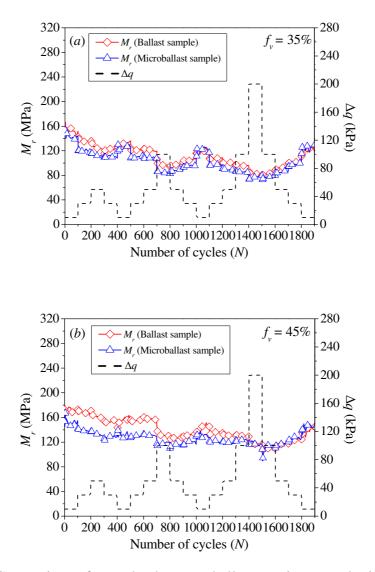


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0.001 - 0.01 (20% of the particles)

0.0003 - 0.01 (80% of the particles)

0.0009 - 0.25 0.0009 - 0.50 0.063 - 0.50 0.16 - 0.63 0.25 - 1 0.32 - 2 0.32 - 2