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1 **Effect of fine particles on the hydraulic behavior of interlayer**
2 **soil in railway substructure**

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17 **Abstract:**

18 The ancient railway substructure in France was built by emplacing ballast directly on sub-grade.
19 Over years of operation, the inter-penetration of ballast and sub-grade created a soil layer
20 between them. Under different conditions, this naturally formed layer, namely interlayer, can
21 contain different quantities of fine particles, becoming more or less sensitive to changes in water
22 content. As the water content changes are governed by the hydraulic behavior of interlayer soil,
23 assessing the influence of fine particles content on the hydraulic behavior of interlayer soil is
24 important. To this end, the hydraulic behavior of an interlayer soil taken from S nissiat (near
25 Lyon, France) was investigated using two infiltration columns, a large-scale column equipped
26 with tensiometers and TDR for suction and volumetric water content measurements, respectively,
27 and a smaller column equipped with high capacity tensiometers only. Different fines contents
28 were considered and wetting-drying cycles were applied to the soil specimens. The hydraulic
29 conductivity was determined by applying the instantaneous profile method. The results obtained
30 showed that i) hysteresis exists for both the soil water retention curve and the hydraulic
31 conductivity changes with suction; ii) the effect of wetting-drying cycles is insignificant; iii)
32 adding 10% of fine particles to the natural interlayer soil changes the soil water retention curve
33 but does not induce significant changes in hydraulic conductivity; iv) the hydraulic conductivity
34 of interlayer soil with 10% of fine particles added is close to that of soil sieved at 2 mm,
35 suggesting that the hydraulic conductivity of interlayer soil is mainly governed by fine particles
36 through suction effect.

37 **Keywords:** railway substructure; interlayer soil; fines content; instantaneous profiles method;
38 hydraulic conductivity.

39

40 ***Introduction***

41 Many railway lines over the world have been in operation for more than one hundred years. In
42 France, the ancient lines represent 94% of the whole railway network. As opposed to the new
43 lines, the ancient ones were constructed by direct installation of ballast onto sub-grade without
44 any separation layer. Over years of operation and with the increasing traffic, load, and speed of
45 train, there are more and more problems related to the stability, loss of strength of substructure. A
46 number of studies have been conducted to assess the state of substructure and to develop
47 adequate maintenance methods (Trinh 2011; Duong et al. 2013; Cui et al. 2013). It was found
48 that one of the particularities of ancient substructure is the presence of a soil layer namely
49 interlayer that has been created mainly by interpenetration of ballast and fine particles of sub-
50 grade.

51 In France, it has been decided recently to renew the ancient railway network. During the
52 renewal, the interlayer will be kept as part of the substructure thanks to its high mechanical
53 resistance related to its high dry unit mass (2.4 Mg/m^3 at the S nissiat site, according to Trinh et
54 al. 2011) reached by natural dynamic compaction corresponding to the circulation of trains.
55 However, the mechanical behavior of interlayer soil can show a large variability, depending on
56 the proportion of fine particles contained in it. A number of studies (Babic et al. 2000; Pedro
57 2004; Naeini and Baziar 2004; Kim et al. 2005; Verdugo and Hoz 2007; Cabalar 2008; Seif
58 El Dine et al. 2010; Ebrahimi 2011; Anbazhagan et al. 2011; Trinh et al. 2012) showed that the
59 mechanical behavior of soil containing a large proportion of fines is strongly influenced by the
60 water content. As the water content changes are governed by the hydraulic behavior of soil, it
61 appears important to assess the influence of fine particles content on the hydraulic behavior of
62 interlayer soil.

63 Moreover, in field conditions, the interlayer soil normally undergoes the effect of
64 wetting/drying cycles related to climatic changes. These wetting/drying cycles may induce
65 changes in soil micro-structure, thereby changing the soil hydraulic properties. Therefore, it
66 appears also important to investigate the effect of wetting/drying cycles on the hydraulic
67 conductivity.

68 To the authors' knowledge, the effects of fines content and wetting/drying cycles on the
69 unsaturated interlayer soil have not been investigated yet. In the present work, laboratory tests
70 were performed using a large-scale infiltration column (300 mm in diameter) and a small-scale
71 infiltration column (50 mm in diameter), and the instantaneous profile method was used to
72 determine the hydraulic conductivity of soil. Both wetting and drying paths were performed and
73 different fines contents were considered: natural interlayer soil (ITL_0), natural interlayer soil with
74 10% of sub-grade added (ITL_{10}), fine-grained soil prepared by passing ITL_{10} through a 2 mm
75 sieve (*Fines*). The results enable the assessment of the effects of fine particles and wetting/drying
76 cycles.

77 ***Materials studied***

78 The soils (both the interlayer soil and sub-grade) were taken from the railway site S nissiat
79 (North-West of Lyon, France). Mineralogy analysis reveals that the interlayer soil is a mixture of
80 materials that come from the construction and maintenance (broken stones, gravel, sand, etc) of
81 tracks, the aging process of track components and the sub-grade. It also showed that the fine
82 particles in the interlayer soil mainly come from the sub-grade. The main geotechnical properties
83 of interlayer soil and sub-grade are presented in Table 1. The results show that the sub-grade is
84 high-plasticity silt. More details about the characterization of the interlayer soil can be found in
85 Trinh et al. (2011).

86 In order to study the effect of fines contents on the hydraulic behavior of interlayer soil, a
 87 quantity of sub-grade representing 10% of interlayer soil by dry mass was added into the
 88 interlayer soil to form a soil with a higher content of fines: ITL_{10} . The grain size distribution
 89 curves of the natural interlayer soil (ITL_0) and ITL_{10} are presented in Fig. 1.

90 It is worth noting that the migration of fines into ballast is recognized as one of the
 91 mechanisms for fouled ballast (Ayres 1986; Selig and Waters 1994; Alobaidi and Hoare 1996;
 92 1998; Ghaotara et al. 2006; Mayoraz et al. 2006; Huang et al. 2009; Giannakos 2010; Fortunato
 93 et al. 2010; Indraratna et al. 2011; Ebrahimi 2011; Sussmann and Chrismer 2012). Even though
 94 the interlayer soil studied here is different from the fouled ballast by nature, in order to compare
 95 with the classification of fouled ballast, two parameters for fouled ballast are adopted here: the
 96 fouling index FI (Selig and Water 1994) and the relative fouling ratio R_{b-f} (Indraratna et al. 2011).
 97 FI is defined as:

98

99 [1]
$$FI = P_4 + P_{200}$$

100 where P_4 and P_{200} are percentages of ballast passing through sieves N° 4 (4.75 mm) and N° 200
 101 (0.075 mm), respectively.

102 R_{b-f} is the weighted ratio of the dry mass of fouling particles M_f (passing through 9.5 mm
 103 sieve) to the dry mass of ballast M_b (particles retained in 9.5 mm sieve):

104 [2]
$$R_{b-f} = \frac{M_f \times \frac{G_{s-b}}{G_{s-f}}}{M_b} \times 100\%$$

105 where G_{s-f} , G_{s-b} are specific densities of fouling particles and ballast, respectively.

106 The values of the two indexes for ITL_0 and ITL_{10} are presented in Table 2. According to
107 the classification, both ITL_0 and ITL_{10} are “highly fouled”. Because ITL_{10} is classified in the
108 highest category of fouling, it was decided to not adding more fine particles to the interlayer soil.

109 To better evaluate the effect of fines on the hydraulic behavior of interlayer soil, the
110 hydraulic conductivity of pure fine particles was also determined. For this purpose, ITL_{10} was
111 sieved at 2 mm to obtain the fine part (namely *Fines*). The grain size distribution curve of *Fines*
112 is also presented in Fig. 1.

113 ***Experimental methods***

114 The interlayer soil was tested in a large-scale infiltration column (Fig. 2). The column (300 mm
115 in diameter and 600 mm in height) is equipped with five water content sensors (TDR1 to TDR5)
116 and five tensiometers for measuring pore-water pressure (T1 to T5) arranged at various elevations
117 along the column ($h = 100, 200, 300, 400$ and 500 mm from the bottom of the soil specimen).
118 The working pressure range of the tensiometers is from 100 kPa to -85 kPa. The accuracy of the
119 TDR used is $\pm 2\%$ and that of the tensiometer is ± 0.5 kPa. At each instrumented height, as the
120 area occupied by the sensors is just 6.8% of the total apparatus section area, the influence of the
121 sensors installation on water transfer is expected to be insignificant.

122 For the ITL_{10} specimen preparation, water and fine particles were added to the dry natural
123 interlayer soil to reach the target water content and fine particles content, and a large mixer was
124 used to homogenize the material. For the ITL_0 specimen preparation, only desired quantity of
125 water was added to the dry natural interlayer soil. After mixing, the wet materials were stored in
126 hermetic containers for at least 24 h for moisture homogenization. Soil compaction was
127 conducted using a vibrating hammer in six layers of 0.10 m each at a dry unit mass of 2.01

128 Mg/m^3 . The soil for each layer has a same composition as that of the whole sample. Big particles
129 and fine particles were rearranged before the compaction in order to ensure the sample
130 homogenization. Prior to the compaction of the subsequent layer, a TDR probe and a metal rod of
131 25 mm diameter were placed on the compacted layer.

132 Once the soil specimen was prepared, water was injected from the bottom and it flowed
133 out from the outlet after about half an hour. After saturation of the sample, the metal rods were
134 removed and the tensiometers were installed. This protocol was adopted to avoid damaging the
135 tensiometers during the compaction and also any cavitation due to possible high suction in the
136 column. More details about the large-scale infiltration column can be found in Duong et al.
137 (2013).

138 The TDR is an indirect measurement method and several authors reported that the
139 calibration curve depends on the soil texture, unit mass, mineralogy, fines content and particle
140 size (Jacobsen and Schjønning 1993; Stolte et al. 1994; Côté and Roy 1998; Gong et al. 2003;
141 Schneider and Fratta 2009). It is therefore necessary to determine the specific calibration curve
142 for each soil studied. For the natural interlayer soil (ITL_0), a relationship between volumetric
143 water content (θ) and the dielectric constant K_a was established by Duong et al. (2013). As the
144 soil composition in ITL_{10} is different from ITL_0 , another relationship was needed. This was
145 determined separately with a lower specimen of 200 mm at the same unit mass in the same
146 column. One TDR sensor was placed in the middle of the sample. Water was added on the
147 surface of the soil specimen to achieve the desired water content. Once the TDR gave a steady
148 response (after about 8 hours), the water content was considered as being uniform within the
149 sample and the value of dielectric constant K_a was recorded. This operation was repeated until
150 the specimen reached full saturation (with 1 cm water on the soil surface). All the TDR sensors

151 were calibrated in the same fashion. The results obtained on the five sensors are similar. Fig. 3
152 presents the calibration curve of TDR for ITL_{10} along with the fitting calibration curve for ITL_0
153 presented in Duong et al. (2013) at the same dry unit mass (2.01 Mg/m^3). It can be observed that
154 the curve for ITL_0 lies below the curve for ITL_{10} , indicating a clear effect of soil texture. The
155 following equation (based on the model of Topp et al. 1980) was used to fit the experimental data
156 for ITL_{10} :

$$157 \quad [3] \quad \theta = -4.16 \times 10^{-5} \times K_a^3 + 2.11 \times 10^{-3} \times K_a^2 - 2.36 \times 10^{-2} \times K_a + 0.17$$

158 The infiltration tests were conducted in two wetting/drying cycles. After installation of the
159 tensiometers, the saturation of soil column was completed (Saturation 1). This wetting stage was
160 followed by a draining stage (Drainage 1). Water was allowed to drain out through the bottom
161 valves by keeping a constant water level at the bottom of soil sample using an external water
162 source. The first wetting/drying cycle finished by a stage of evaporation (Evaporation 1) where
163 the top cover of the column was removed to allow soil water evaporation. A fan was used to
164 accelerate the evaporation process. The evaporation stage ended when the suction value indicated
165 by tensiometer T5 ($h = 500 \text{ mm}$) was about 60 kPa (higher suction would lead to cavitation). A
166 second wetting-drying cycle was applied following the same procedure (Saturation 2, Drainage 2
167 and Evaporation 2). Before the second drainage, the hydraulic conductivity in saturated state was
168 also measured by applying a constant water head of 0.61 m. The hydraulic gradient was 1.
169 According to the Tennakoon et al. (2012), hydraulic gradient smaller than 4 can be considered as
170 low enough to ensure the Darcy's law. Note that the experimental procedure with saturation from
171 the bottom and evaporation from the top is also recommended in ASTM standard (ASTM 2010).
172 During the measurement of hydraulic conductivity under saturated condition, the volume of water
173 injected increased linearly with a rate of about 50 cm^3 per minute.

174 The unsaturated hydraulic conductivity of *Fines* was determined using a small-scale
 175 infiltration column of 50 mm in diameter and 200 mm in height (Munoz et al. 2008). Its
 176 schematic view is shown in Fig. 4. Suction measurements were performed by four high-capacity
 177 tensiometers (Cui et al. 2008) installed at 40, 80, 120 and 160 mm height from the base of the
 178 sample. The accuracy of this tensiometer is ± 1 kPa. The soil was statically compacted in the
 179 column in four layers of 50 mm each. Once the compaction was completed, the tensiometers
 180 were installed. In order to ensure a good contact between the soil sample and the tensiometers, a
 181 paste of sub-grade was applied on the surface of the ceramic of tensiometers.

182 The dry unit mass and water content of *Fines* were taken equal to those of fine particles
 183 contained in the sample of interlayer soil. Using the illustration shown in Fig. 5, these two
 184 parameters can be calculated as follows:

$$185 \quad [4] \quad \rho_{d,f} = \frac{M_{s,f}}{V_f} = \frac{M_s - M_{s,b}}{V - V_{s,b}} = \frac{(1-m)\rho_d V}{V - \frac{m}{\rho_{s,b}} \rho_d V} = \frac{(1-m)\rho_d \rho_{s,b}}{\rho_{s,b} - m\rho_d}$$

$$186 \quad [5] \quad w_f = \frac{M_w}{M_{s,f}} = \frac{M_s w}{M_s - M_{s,b}} = \frac{w}{1-m}$$

187 where M , M_w , M_s are the total mass, mass of water and mass of solid particles, respectively; V , V_w ,
 188 V_s are the total volume, volume of water and volume of solid particles respectively; ρ_d , ρ_s are the
 189 dry unit mass of the specimen and unit mass of solid particles, respectively; the subscripts f and b
 190 stand for particle smaller and larger than 2 mm, respectively; m is the percentage of particles
 191 larger than 2 mm.

192 Based on the grain size distribution curve, a value $m = 0.67$ was determined. From Eqs (2)
 193 and (3), a value of 1.33 Mg/m^3 was obtained for the dry unit mass of *Fines*.

194 The test procedure followed for the small-scale infiltration column was akin to that for the
195 large-scale one. After the suction stabilization, the sample was saturated from the bottom
196 (Saturation 1). After completion of saturation, an external water source was connected to the
197 bottom in order to ensure a constant water level after the drainage. The top cover was then
198 removed allowing water evaporation from the soil surface (Evaporation 1). When suction at 160
199 mm reached about 400 kPa, Evaporation 1 was stopped to avoid cavitation of the tensiometers. A
200 second wetting-drying cycle was applied by following the same procedure as in the first cycle
201 (Saturation 2 and Evaporation 2).

202 Unlike the large-scale column where both suction and water content were monitored, the
203 small-scale column has only suction monitored. To obtain the water content changes during
204 infiltration, the soil-water retention curve (SWRC) was needed. The water retention curve (WRC)
205 of compacted *Fines* was determined separately using the device presented in Fig. 6. The soil was
206 first compacted inside an oedometer cell (the dimensions of the soil specimen are 50 mm in
207 diameter and 20 mm in height). The suction of the specimen was monitored by a high-capacity
208 tensiometer fixed at the bottom of the cell. A light aluminum piston of 50 mm diameter was
209 placed on the specimen to ensure the good contact between soil and tensiometer. The piston
210 induced a vertical stress of 1.8 kPa and its influence was believed to be negligible. For the
211 monitoring of soil water content, the whole system was placed on a balance having an accuracy
212 of ± 0.01 g. The mass change indicated the quantity of water added or evaporated. More details
213 about this cell can be found in Le et al. (2011) and Munoz-Castelblanco et al. (2012). Wetting
214 was conducted by adding a small quantity of water on the upper face of the sample, while drying
215 was conducted by allowing soil water evaporation from the upper surface without the piston on it.
216 Once the desired water content was reached, the piston was put on the soil surface and the final
217 suction was recorded. This method was also discussed by Cunningham et al. (2003); Toker et al.

218 (2004); Loucenço (2008); Loucenço et al. (2011); Toll et al. (2012) and Munoz-Castelblanco et
219 al. (2012).

220 For the large-scale column, both suction and water content profiles were obtained
221 directly. For the small-scale column, the suction profiles were obtained directly while the water
222 content profiles were determined through the SWRC. The instantaneous profile method (Daniel
223 1982; Delage and Cui 2001; Bruckler et al. 2002; Cui et al. 2008; Ye et al. 2009) was then
224 applied for the determination of hydraulic conductivity for each soil. Note that this method is
225 based on the generalized Darcy's law. The hydraulic gradient is determined by considering the
226 slope of suction isochrones and the water volume passing through a given section between times t
227 and $t+dt$ is used for calculating the water flux.

228 ***Experimental results***

229 The results of ITL_0 were presented in Duong et al. (2013). Here only the results of ITL_{10}
230 and *Fines* are presented in detail, and the results of ITL_0 are only used for comparison.

231 Fig. 7 presents changes in pore water pressure and volumetric water content versus time
232 during Drainage 1 and Evaporation 1 for ITL_{10} . From the saturated state where the volumetric
233 water content reached 22-25%, water drained out through the bottom valves and subsequently the
234 volumetric water content decreased to 15 – 17% at the end of the draining stage for all the TDR
235 sensors except that at $h = 200$ mm (Fig. 7b). At this moment, the pore water pressure was in the
236 range from 0 to -4 kPa (Fig. 7a). Drainage 1 finished after more than 1 day. During Evaporation
237 1, the pore water pressure given by the tensiometer at $h = 500$ mm decreased quickly while small
238 changes were observed at other levels (Fig. 7a). This is consistent with the values of volumetric

239 water content: the value at $h = 500$ m decreased significantly since the beginning of Evaporation
240 1 while those at other levels show slight changes (Fig. 7b).

241 During Saturation 2, the external water source was set at a level applying a water pressure
242 of 6.1 kPa to the bottom of sample. The results obtained show that less than one hour was needed
243 to re-saturate the soil specimen (Fig. 8). The changes were not significant for T1 to T4 (small
244 suction value), while those of T5 at $h = 500$ mm are quite significant (Fig. 8a). Furthermore, the
245 suction changes in Fig. 8a are consistent with those of volumetric water content in Fig. 8b. At the
246 end of this stage when the pore water pressure became positive at all levels, the 5 tensiometers
247 indicated the values corresponding to the water head at each level (5.5 kPa, 4.5 kPa, 3.5 kPa, 2.0
248 kPa and 1.0 kPa for T1 to T5, respectively). The volumetric water content also reached the values
249 of near saturated state (corresponding to the degree of saturations ranging from 87.5% to 100%).

250 The results obtained during Drainage 2 and Evaporation 2 are presented in Fig. 9. During
251 the first two days, water inside the column was connected to the outside water source having a
252 level decreased in steps of 50 mm from $h = 550$ to 50 mm in order to verify the response of the
253 sensors. Each step was kept for 1 hour. At the end of Drainage 2, the outside water source was set
254 at $h = 50$ mm and Evaporation 2 started. During the drainage, the volumetric water content
255 decreased quickly while the changes of suction were much slower. As during Evaporation 1, the
256 pore water pressure and the volumetric water content values at $h = 500$ mm decreased
257 significantly while the others remained almost constant. Once again, the changes of suction and
258 volumetric water content are consistent for different levels: the closer to the evaporation surface,
259 the higher the suction (Fig. 9a) and the smaller the volumetric water content (Fig. 9b) (except for
260 $h = 200$ mm).

261 The inconsistent data given by the TDR sensor at $h = 200$ mm (see Fig. 7, Fig. 8, Fig. 9)
262 is related to the deficiency of this sensor. Indeed, some additional calibrations were conducted
263 after the test, and the results showed some inertia of this TDR sensor: in the full range from 0 to
264 100%, no difference with other sensors was observed; however, in a limited range, a clear
265 difference was identified. Thereby, the results by this sensor were not considered in further
266 analysis.

267 The data of suction and volumetric water content recorded allowed the WRC of the
268 interlayer soil to be determined. The results are presented in Fig. 10 with three paths
269 corresponding to Drainage 1-Evaporation 1 (Drying 1), Saturation 2 (Wetting 2) and Drainage 1-
270 Evaporation 2 (Drying 2). It can be observed that the results of two drying processes are close. In
271 contrast, the result of wetting path lies above. Note that because the minimum recording interval
272 of TDR was every minute and the wetting process took place very quickly; there are less data for
273 the wetting path. The model of van Genuchten (1980) was used to fit the experimental data with
274 the parameters presented in Table 3.

275 The hydraulic conductivity versus suction is presented in Fig. 11, including the hydraulic
276 conductivity measured at saturated state, equal to 1.67×10^{-5} m/s. It can be seen that the results for
277 the two drying paths are similar, suggesting negligible microstructure changes. For the wetting
278 path, all results lie above those of the drying paths, illustrating a clear phenomenon of hysteresis.
279 The models of van Genuchten (1980) and Brooks-Corey (Brooks and Corey 1964; Stankovich
280 and Lockington 1995) were used for fitting the data of both drying and wetting paths using the
281 least squares method (see Fig. 11). The parameters determined are presented in Table 4 .

282 Fig. 12 presents the comparison between the WRC of ITL_0 and ITL_{10} for the drying path
283 (the parameters of van Genuchten's model (1980) used for fitting the experimental data are

284 presented in Table 3). Note that for clarity, the scattered data at low suction are not included. The
285 WRC of ITL_0 is beneath the WRC of ITL_{10} , suggesting that at a given suction, ITL_{10} has a higher
286 water content than ITL_0 . This appears normal because with the same dry unit mass, the higher the
287 fines content, the higher the retention capacity.

288 The comparison of hydraulic conductivity between ITL_0 and ITL_{10} is presented in Fig. 13.
289 In the saturated state, the two soils have almost the same value: 1.67×10^{-5} m/s for ITL_{10} and
290 1.75×10^{-5} m/s for ITL_0 . Both values are lower than the critical value proposed by Selig and
291 Waters (1994) for the railway substructures. In unsaturated state, even the data are scattered for
292 the two soils, an identical trend can be identified: the hydraulic conductivity is decreasing with
293 the increase of suction. Moreover, the average value for ITL_{10} is slightly higher than that for ITL_0 ,
294 suggesting a slightly greater hydraulic conductivity for ITL_{10} . On the whole, the difference
295 between the hydraulic conductivity results of two soils is less evident than the difference between
296 the SWRC results.

297 The results obtained from the small-scale infiltration test on *Fines* are shown in Fig. 14 to
298 Fig. 18. After installation of the tensiometers, a period of 18 hours was needed to reach the
299 suction equilibrium at 70 – 83 kPa (Fig. 14). This difference in final suctions at different levels
300 was mainly related to the soil heterogeneity. The corresponding degree of saturation was 43%.
301 From this initial state, the soil was first re-saturated by injecting water from the bottom with a
302 constant water head of 0.7 kPa. The suction at the lowest level changed first, followed by the
303 suctions at higher levels (Fig. 15). Ten minutes after the water injection, water appeared on the
304 upper surface and suctions at all level reached zero, indicating the full saturation of the soil
305 specimen.

306 After Saturation 1, water was drained out to an outside water source and the water level
307 was maintained at $h = 0$. Afterwards, Evaporation 1 took place. The results obtained are shown in
308 Fig. 16. Fifteen hours later, the pore water pressure measured at $h = 160$ mm (40 mm below the
309 soil surface) started to decrease and reached -300 kPa at 57 hours. The changes in water pressures
310 measured by other tensiometers were less significant.

311 Saturation 2 took place right after Evaporation 1. The results obtained during this second
312 wetting stage are shown in Fig. 17. Less than 2 minutes was required for the pore water pressure
313 at $h = 160$ mm to come back from -300 kPa to about 0. The results obtained during the
314 subsequent drying are shown in Fig. 18 (Evaporation 2). As in the case of Evaporation 1, after 80
315 hours, the pore water pressure at $h = 160$ mm decreased to - 365 kPa while those at other levels
316 did not change significantly.

317 The results from the test in the tensiometer-equipped oedometer are shown in Fig. 19. Fig.
318 19a depicts the suction (negative pore water pressure given by the tensiometer) evolution after the
319 tensiometer installation. The suction increased and reached its stabilization value of 110 kPa after
320 17 hours. This corresponds to the initial state of the soil specimen (21.3 % volumetric water
321 content and 42.6 % degree of saturation). Water was then added into the specimen to follow the
322 wetting path. The variation of the first step of wetting is presented in Fig. 19b. About 35 minutes
323 was needed for suction stabilization. The volumetric water content in this step increased from
324 21.3% to 21.7%. This operation was repeated until the soil reached the near saturated state. Then
325 the drying steps started. Fig. 19c presents the suction stabilization during one drying step. An
326 equilibrium value of 79 kPa was reached after 320 minutes. This suction increase corresponded to
327 a decrease of volumetric water content from 22.2% to 21.6%.

328 The SWRC obtained for *Fines* is shown in Fig. 20. From its initial state, the soil specimen
329 was subjected to wetting up to 70% degree of saturation, followed by drying and finally a second
330 wetting till full saturation. It can be seen that the SWRC obtained during drying lies above that
331 during wetting. The maximum suction value was 390 kPa corresponding to a volumetric water
332 content of 18.9%; it was also close to the maximum suction value in the specimen during the
333 infiltration test, indicating the compatibility of the two tests.

334 This SWRC determined was then used to calculate the changes of volumetric water
335 content in the small-scale infiltration column based on the suction changes presented in Fig. 15 to
336 Fig. 18. Either the drying or the wetting path was used depending on the path followed in the
337 infiltration test. As the second drying path of SWRC was not available, the first drying path was
338 used for calculating the volumetric water content during the second drying path in the infiltration
339 test. Then, based on the profiles of suction and water content, the hydraulic conductivity of
340 compacted *Fines* was calculated. The results are shown in Fig. 21, including the hydraulic
341 conductivity measured at saturated state by applying a constant water pressure of 0.7 kPa:
342 2.6×10^{-6} m/s. Albeit the large data scatter, a clear trend can be observed: as for the interlayer soil,
343 the hydraulic conductivity increased when the suction decreased.

344 It is worth noting that the results obtained for the two drying paths are quite similar. The
345 same conclusion can be drawn for the two wetting paths. The models of van Genuchten (1980)
346 and Brooks-Corey (Brooks and Corey 1964; Stankovich and Lockington 1995) were used to fit
347 the results (Fig. 21), and the parameters determined are presented in Table 4. Comparison
348 between the drying and wetting curves shows that for the 2 wetting/drying cycles, the wetting
349 curves lie always above the drying curves. It is opposed to the SWRC where the wetting curves
350 are normally beneath the drying ones. In addition, the curves of wetting path and drying path of

351 the 1st cycle are close to those corresponding to the 2nd cycle, suggesting no effect of
352 wetting/drying cycles on the hydraulic conductivity. The smallest value of hydraulic conductivity
353 identified is 6×10^{-12} m/s corresponding to a suction value of 242 kPa, while the highest one is
354 2.6×10^{-6} m/s corresponding to the saturated state.

355 Fig. 22 depicts the comparison of SWRC between *ITL₁₀* and *Fines* in the plane of degree
356 of saturation versus suction. The two curves start from almost the same point - around 97%
357 degree of saturation and 1.7 kPa suction. From 3 kPa suction, the WRC of *ITL₁₀* starts to separate
358 from the WRC of *ITL₀*. The two curves are parallel (for drying path) from 10 kPa suction. The
359 curves of *ITL₁₀* stop at 71 kPa while the curves of *Fines* stop at 389 kPa due to the different
360 capacities of the tensiometers used for the two soils. The gap between two curves is about 10% of
361 degree of saturation at the end of the curve for *ITL₁₀*.

362 In Fig. 23, the hydraulic conductivity of *ITL₁₀* and *Fines* is plotted versus suction. It can
363 be observed that the wetting and drying curves of the interlayer soil are quite close to those of
364 *Fines*, suggesting that the hydraulic conductivity of the interlayer soil is mainly governed by the
365 hydraulic conductivity of the fines contained in it. In other words, water transfer in the interlayer
366 soil takes place mainly through the network of pores between fine particles, coarse elements like
367 ballast behaving as inert materials. This is confirmed by the hydraulic conductivity values at
368 saturated state: similar values were identified - 1.67×10^{-5} m/s for *ITL₁₀* against 2.6×10^{-6} m/s for
369 *Fines*.

370 From a practical point of view, Fig. 23 shows that to determine the hydraulic conductivity
371 of interlayer soils, it is not necessary to use large-scale experimental devices to match the soil
372 grain size; smaller devices can be used to determine their hydraulic conductivity by testing the
373 fine particles only, provided that equivalent dry density is accounted for.

374 **Discussions**

375 Fig. 11 and Fig. 21 show the uncommon phenomenon of hysteresis observed in hydraulic
376 conductivity of *ITL₁₀* and *Fines*, respectively. The curve of wetting path lies above the curve of
377 drying path. The same phenomenon was observed by Wayllace and Lu (2011). The following
378 interpretations can be attempted.

379 Due to the different kinetic between the fast liquid transfer in wetting and the long vapor
380 transfer in drying, the time needed for drying was much longer than that for wetting. This
381 phenomenon was also reported by Toker et al. (2004) and discussed by Munoz-Castelblanco et
382 al. (2012). A higher hydraulic conductivity can be expected in that case for wetting path.

383 In the present work, the calculation of hydraulic conductivity was performed based on the
384 suction evolution given by tensiometers. Assuming that in the compacted soils, both macro-pores
385 and micro-pores existed. During wetting, the macro-pores were filled with water more quickly
386 than micro-pores. Moreover, in the micro-pores, there were always air bubbles preventing the
387 total saturation. In contrast, during drying, all pores (micro and macro) participated in the
388 evaporation process. As a result, when water filled the macro-pores, the tensiometers
389 immediately gave the suction changes corresponding to the water flow through the macro-pores,
390 even though the suction in micro-pores would be higher. On the contrary, when water evaporates
391 during drying, the tensiometers gave the suction changes that involve both macro and micro-
392 pores, in a much slower fashion. In other words, the suction measured by the tensiometers was
393 probably under-estimated for wetting paths. Côté and Roy (1998) also reported that one re-
394 saturating stage is not enough to fully saturate a soil sample because of the air bubbles trapped in
395 micro-pores. This can also explain the uncommon hysteresis mentioned before.

396 Poulouvassilis (1969) (cited by Mualem 1976) considered, in a qualitative way, the
397 influence of capillary hysteresis on the hydraulic conductivity based on the concept of
398 independent domain theory. He defined two mechanisms related to hysteresis: (i) water fills pores
399 of larger opening radius in a wetting process than in drying one; (ii) the pores configuration and
400 interconnection may be different for wetting and drying. As a result, the hydraulic conductivity in
401 wetting may be different from that in drying for the same water content. The theory about the
402 difference between the opening radius affecting wetting process and the opening radius affecting
403 drying process is known as the effect of ink-bottle (Bertotti and Mayergoyz 2006; Naumov
404 2009). According to this theory, the pores affecting the wetting curve are larger than the pores
405 affecting the drying curve. As a result, the water transfer is faster during wetting than during
406 drying, implying a higher hydraulic conductivity in the case of wetting.

407 **Conclusions**

408 Infiltration tests were performed on the interlayer soil (ITL_0) and its derived soils - adding 10% of
409 sub-grade to form ITL_{10} and sieving ITL_{10} at 2 mm to form *Fines*. Two wetting/drying cycles
410 were applied for each test. The obtained results allowed the effect of fine particles on the water
411 retention capacity and hydraulic conductivity of interlayer soil to be analyzed.

412 The effect of wetting/drying cycles on hydraulic conductivity was found negligible - the
413 results of the first cycle are quite similar to those of the second cycle, suggesting an insignificant
414 microstructure change by wetting/drying cycles.

415 Hysteresis exists for both the soil water retention curve and the hydraulic conductivity
416 changes with suction. The wetting process was found to be much faster than the drying process,
417 and the hydraulic conductivity during wetting is always higher than that during drying. This can

418 be explained by the effect of ink-bottle and the difference between the water transfer through the
419 network of macro-pores and micro-pores.

420 Adding 10% fine particles to the natural interlayer soil changes the soil water retention
421 curve but does not induce significant changes in hydraulic conductivity. In saturated state, the
422 hydraulic conductivity of natural interlayer soil is 1.75×10^{-5} m/s, while the value of the soil with
423 10% fines added is 1.67×10^{-6} m/s. In unsaturated state, even though the results are little scattered,
424 the results of ITL_{10} are within the variation range of the results of ITL_0 . However, it is worth
425 noting that the mean value of ITL_{10} is slightly greater than that of ITL_0 .

426 The water retention curves of ITL_{10} and *Fines* are different, illustrating an obvious effect
427 of soil texture. On the contrary, in terms of hydraulic conductivity including the values in
428 saturated state, a good agreement was identified between the results of two soils, regardless of the
429 drying or wetting paths. This suggests that water transfer in the interlayer soil takes place mainly
430 through the network of pores between fine particles, coarse elements like ballast behaving as inert
431 materials. From a practical point of view, this finding shows that to determine the hydraulic
432 conductivity of interlayer soils, a device as small as the small-scale infiltration cell can be
433 employed to determine the hydraulic conductivity of interlayer soil by testing the fine particles
434 only, provided that equivalent dry density is taken into account.

435

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615

616 **Table 1: Properties of the soil studied**

Soil	Properties	Value
Interlayer soil (ITL_0)	ρ_s (particles smaller than 2 mm)	2.67 Mg/m ³
	ρ_s (particles larger than 2 mm)	2.68 Mg/m ³
	d_{10}	0.01 mm
	d_{30}	5 mm
	d_{60}	30 mm
	liquid limit w_L (smaller than 100 μm)	40.2%
	plasticity index I_p (smaller than 100 μm)	11.3%
Sub-grade (Fines to create ITL_{10})	liquid limit w_L	57.8%
	plasticity index I_p	24.1%

617

618

619 **Table 2: Fouling state of the interlayer soil**

	Fouling Index	Relative ballast fouling ratio	Fouling category
	FI (-)	R_{b-f} (%)	
ITL_0	45	56	Highly fouled
ITL_{10}	59	72	Highly fouled

620

621 **Table 3: Parameters of the van Genuchten’s model (1980) for the soil water retention curves of interlayer soil**

	Natural interlayer soil (<i>ITL₀</i>)	Interlayer soil with 10% sub-soil added (<i>ITL₁₀</i>)		
Formula	Drying	Drying 1	Wetting 2	Drying 2
$\theta = \theta_r + \frac{\theta_s - \theta_r}{\left[1 + (\alpha h)^n\right]^m}$	$\theta_s = 25$	$\theta_s = 25$	$\theta_s = 22.5$	$\theta_s = 25$
	$\theta_r = 0$	$\theta_r = 10$	$\theta_r = 10$	$\theta_r = 10$
	$\alpha = 4$	$\alpha = 159.62$	$\alpha = 0.19$	$\alpha = 113.45$
	$n = 1.17$	$n = 1.24$	$n = 5.24$	$n = 1.25$
	$m = 0.15$	$m = 0.19$	$m = 0.81$	$m = 0.2$

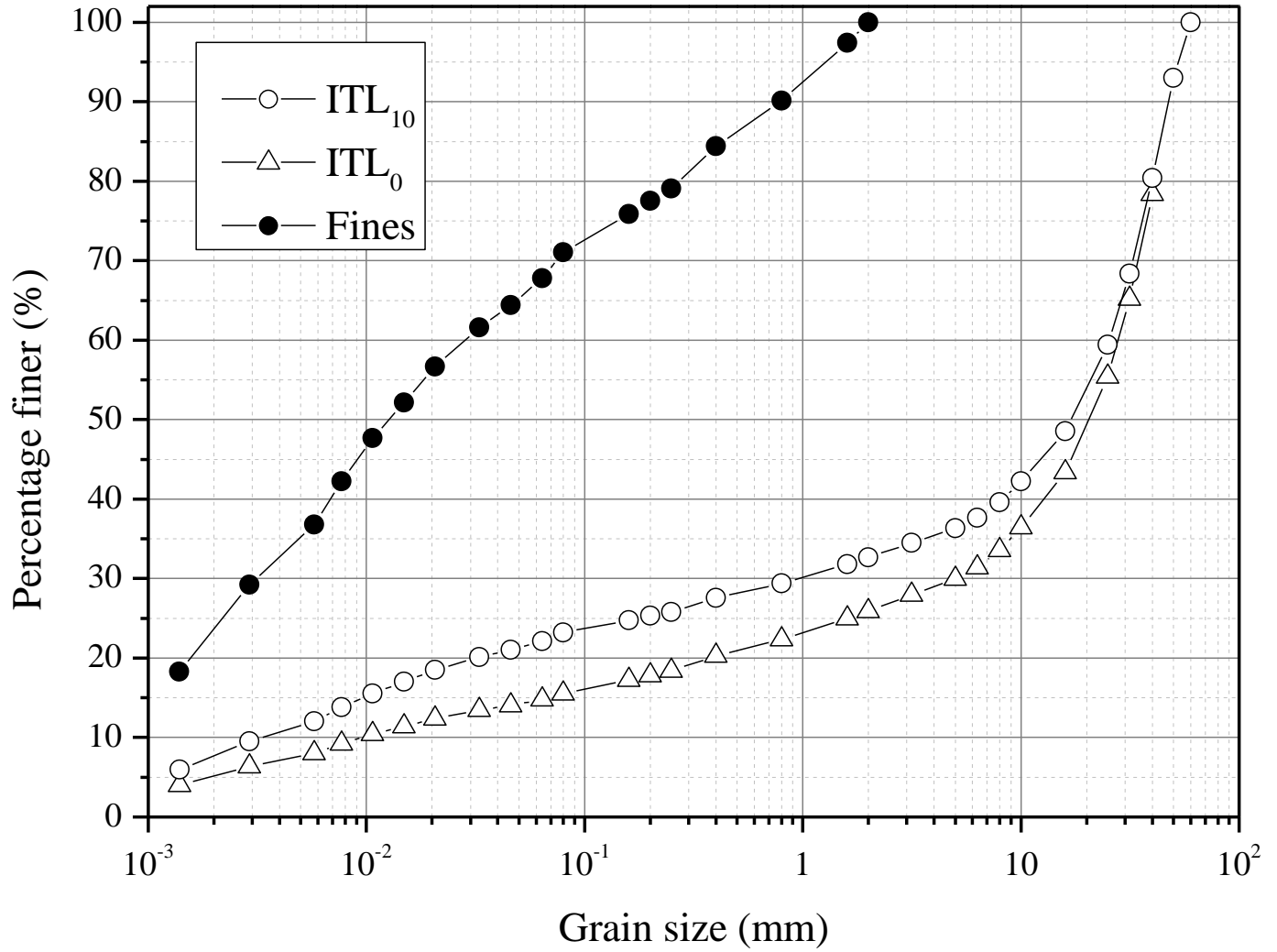
622 *Notes:* θ is volumetric water content (%); θ_s is the volumetric water content at saturated state (%);
 623 θ_r is the residual volumetric water content (%); h is hydraulic head in meter; α , n , m are the
 624 model’s parameters

625 **Table 4: Parameters of the van Genuchten’s model and Brooks-Corey’s model for the hydraulic conductivity**
 626 **of *ITL₁₀* and *Fines***

Model	Formula	Soil	Drying path	Wetting path
van Genuchten	$k = k_s \frac{1 - (\alpha h)^{n-2} \left[1 + (\alpha h)^n\right]^{-m}}{\left[1 + (\alpha h)^n\right]^{2m}}$	<i>ITL₁₀</i>	$\alpha = 7.16$ $n = 2.12$ $m = 0.06$	$\alpha = 0.5$ $n = 2.4$ $m = 0.17$
		<i>Fines</i>	$\alpha = 2.1$ $n = 2.06$	$\alpha = 0.25$ $n = 2.37$

			$m = 0.03$	$m = 0.15$
		For the two soils	$\alpha = 2.2$ $n = 2.1$ $m = 0.05$	$\alpha = 0.3$ $n = 2.7$ $m = 0.16$
Brooks-Corey	$k = k_s \left(\frac{s_a}{s} \right)^{2+3\lambda}$	<i>ITL₁₀</i>	$s_a = 0.3$ $\lambda = 0.05$	$s_a = 4$ $\lambda = 0.1$
		<i>Fines</i>	$s_a = 0.7$ $\lambda = 0.01$	$s_a = 7$ $\lambda = 0.01$
		For the two soils	$s_a = 0.74$ $\lambda = 0.03$	$s_a = 5.4$ $\lambda = 0.02$

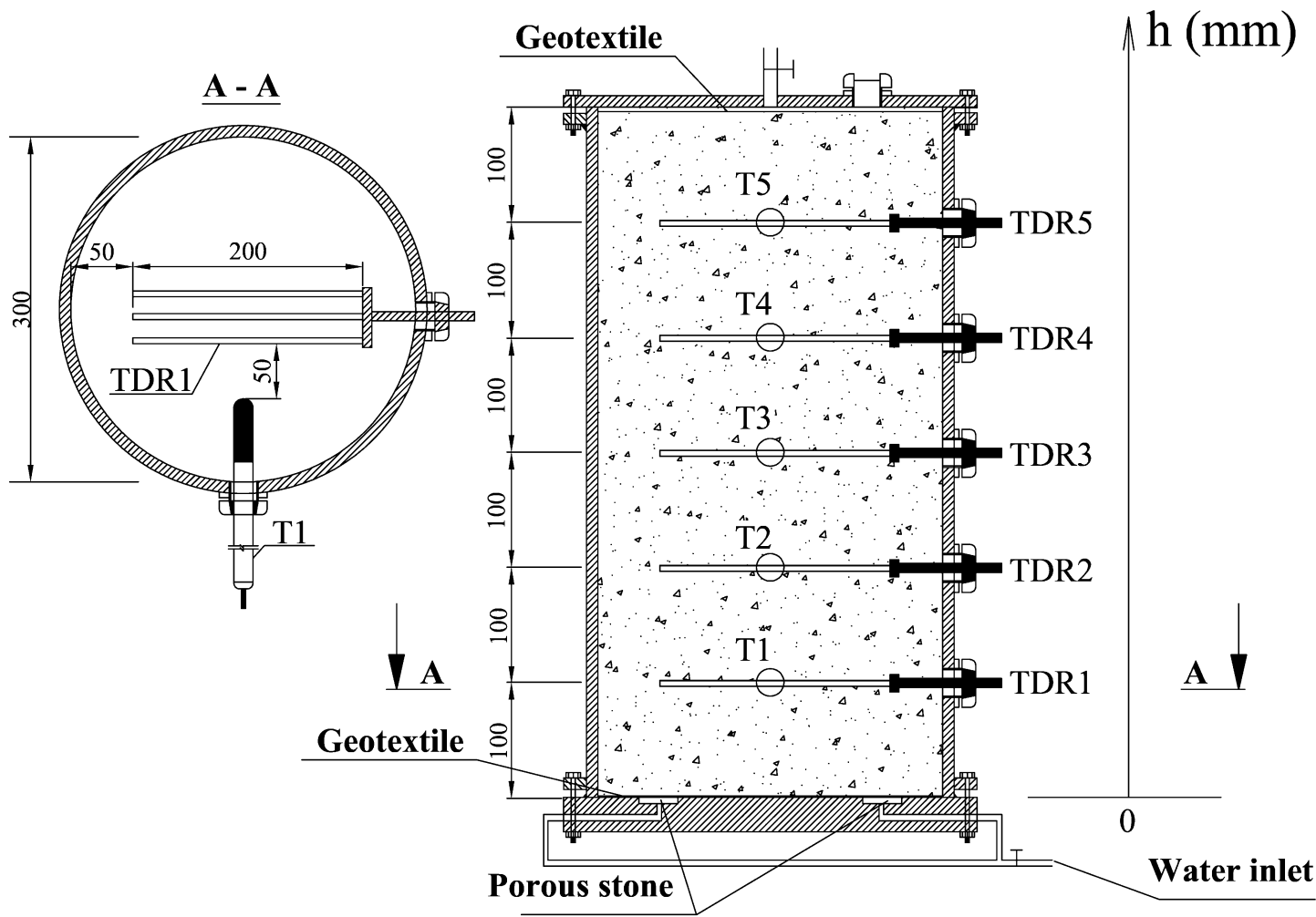
627 *Notes:* k is the hydraulic conductivity; k_s is the hydraulic conductivity in saturated state; h is
628 hydraulic head in meter; s is the suction (kPa); s_a is the air-entry value (kPa); λ , α , n , m are
629 parameters of the models.



630

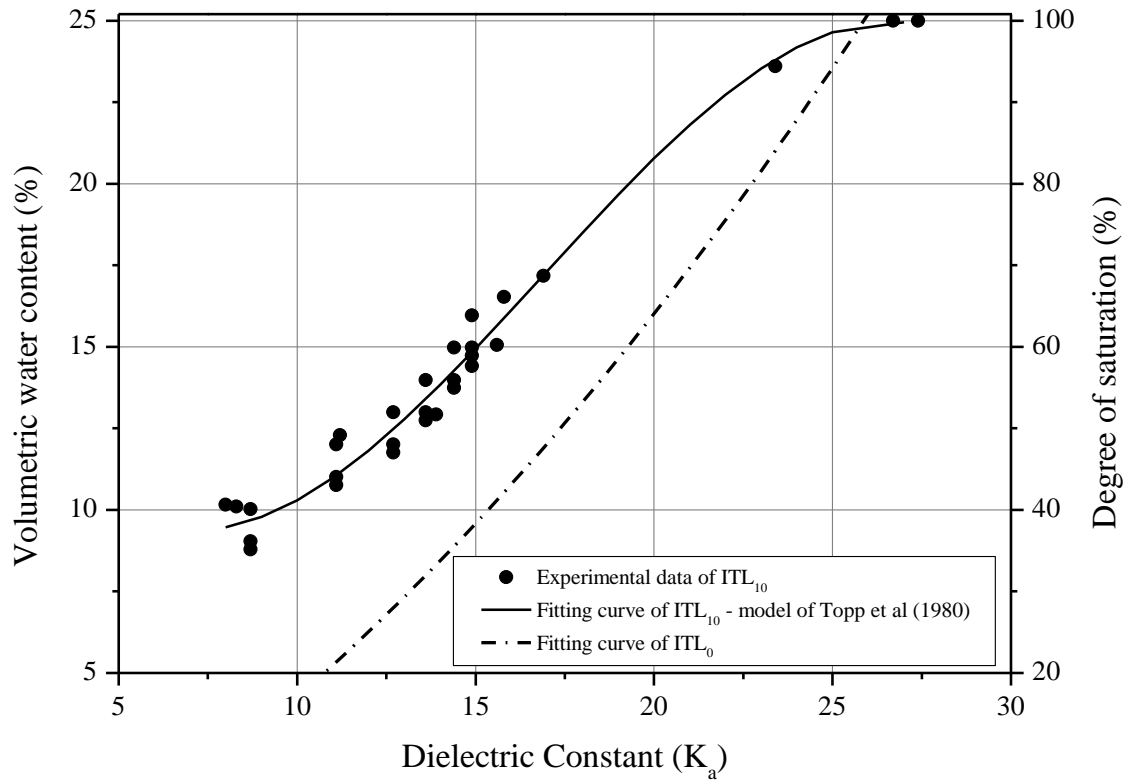
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Fig. 1: Grain size distribution curves of the interlayer soil (ITL_0) and the derived ones (ITL_{10} and Fines)



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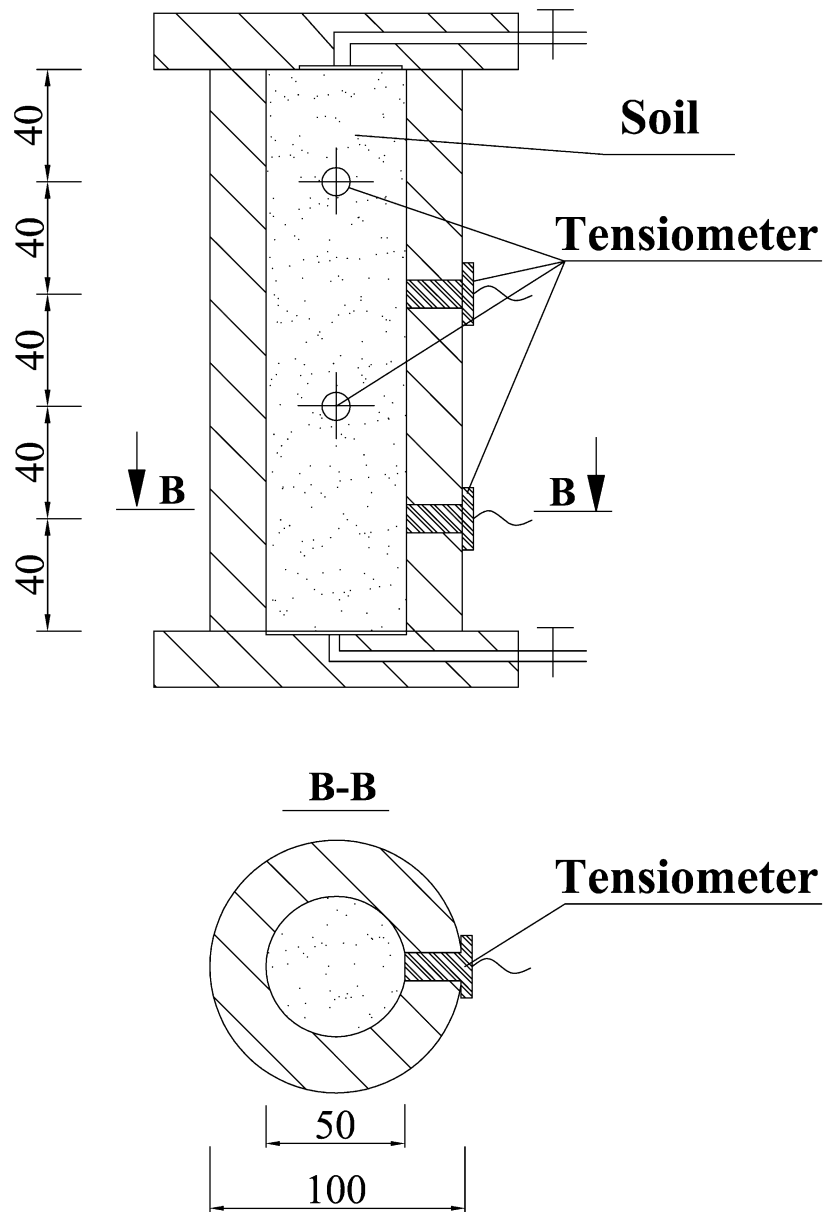
633 Fig. 2: Schematic view of the large-scale infiltration column



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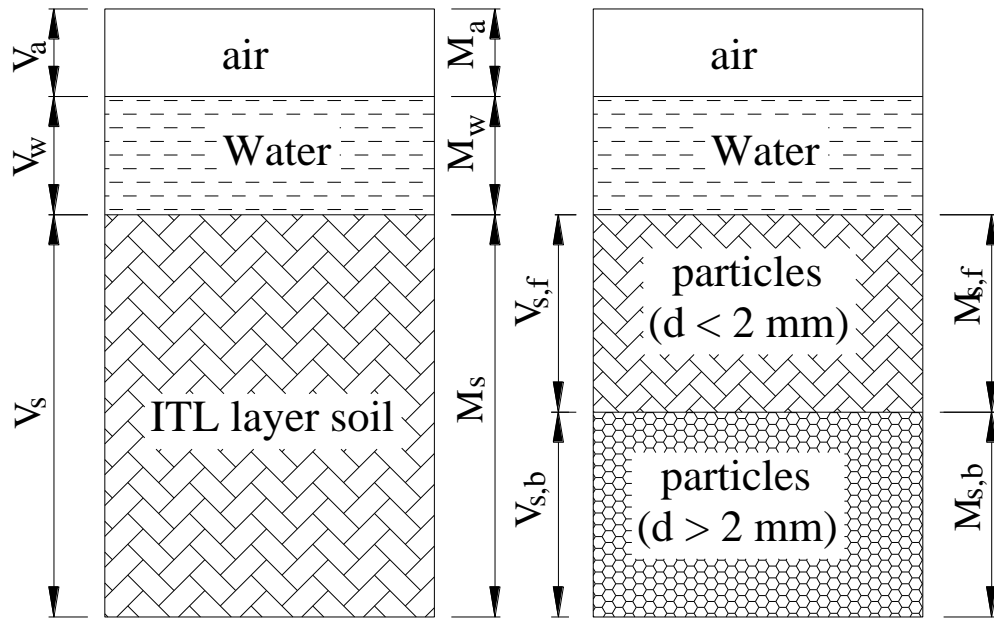
635 **Fig. 3: Calibration curves of the TDRs used for the ITL_{10} and ITL_0 specimens**

636



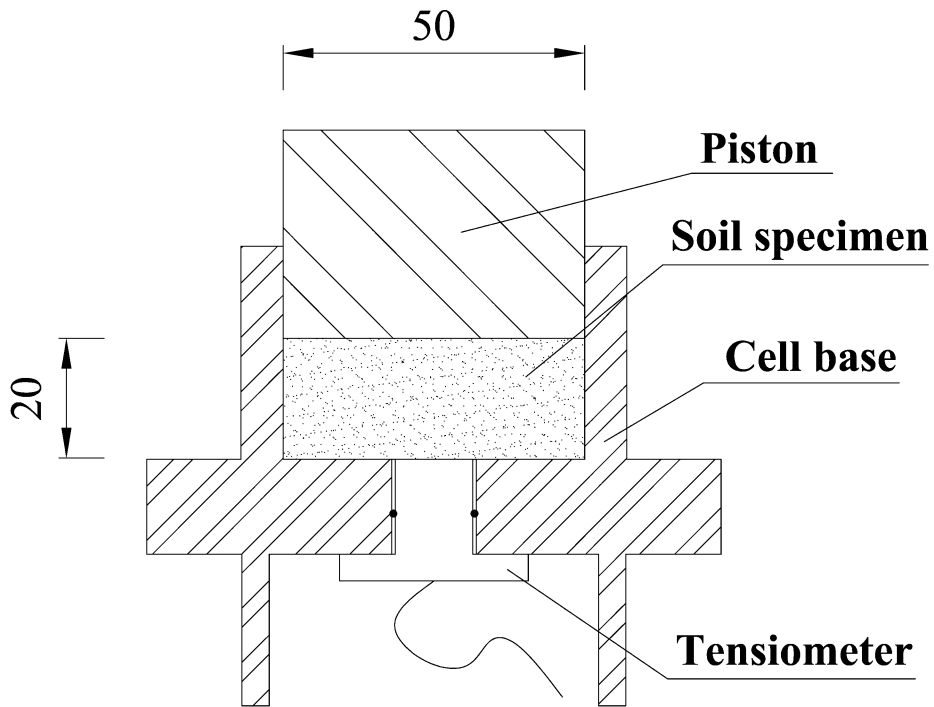
637

638 **Fig. 4: Schematic view of the small-scale infiltration column**



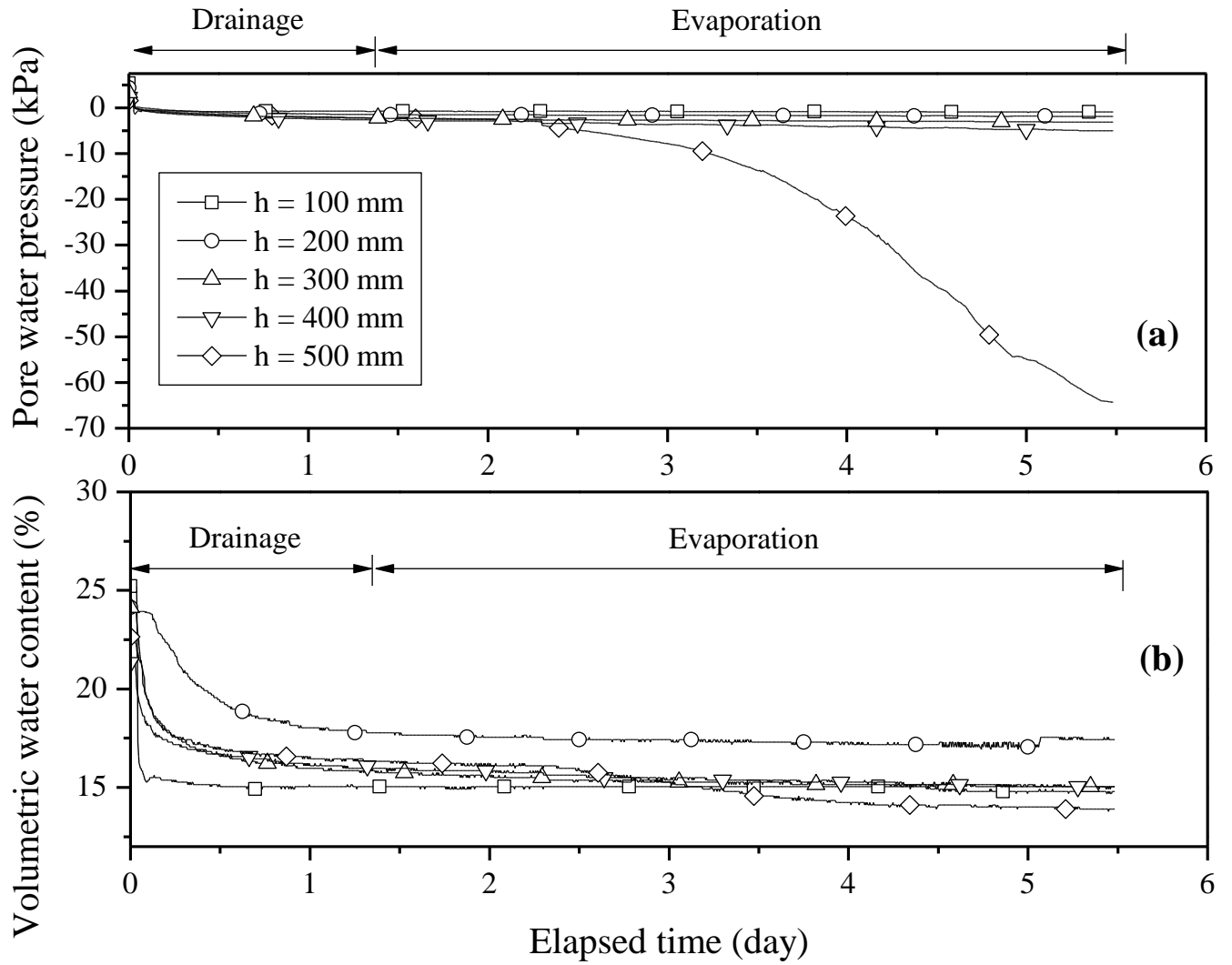
639

640 **Fig. 5: Components of the unsaturated interlayer soil**



641

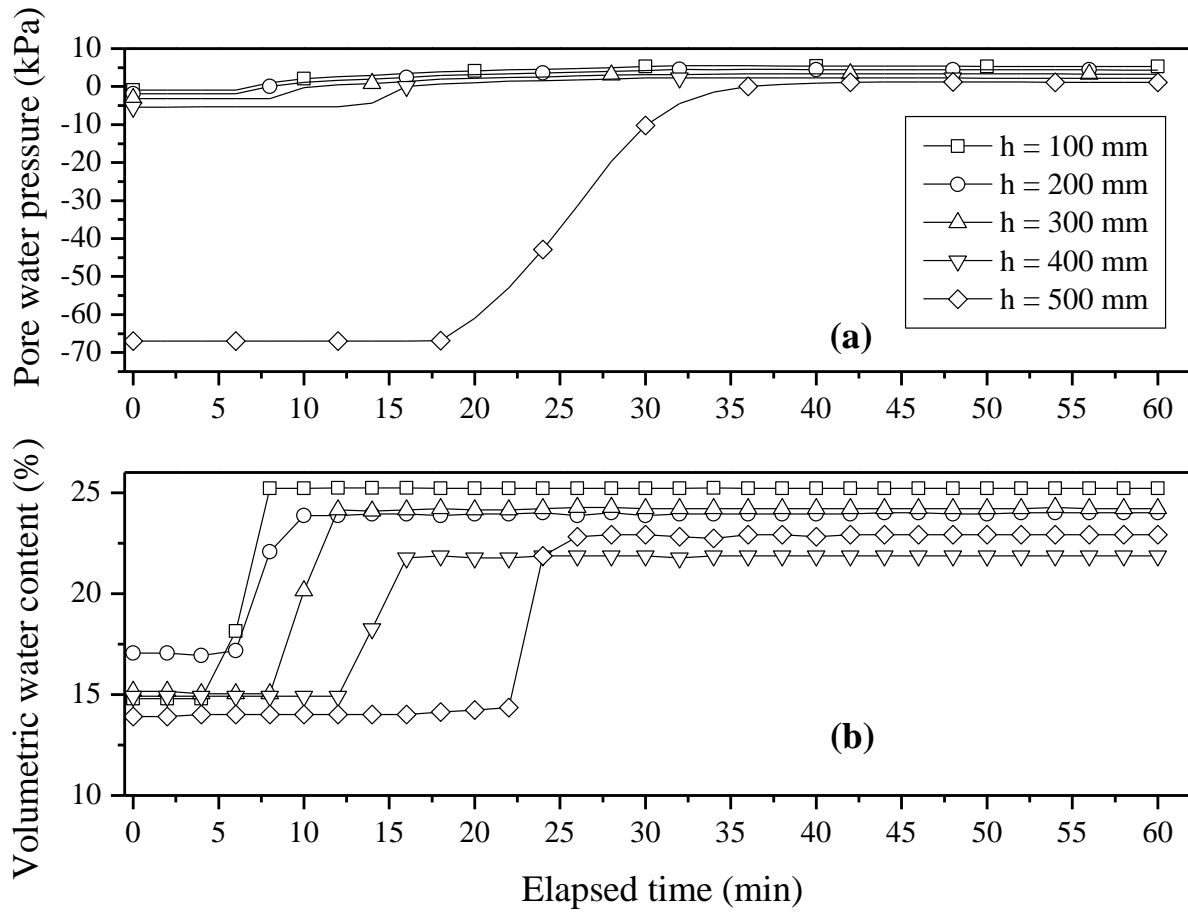
642 **Fig. 6: Device for determining the WRC of Fines**



643

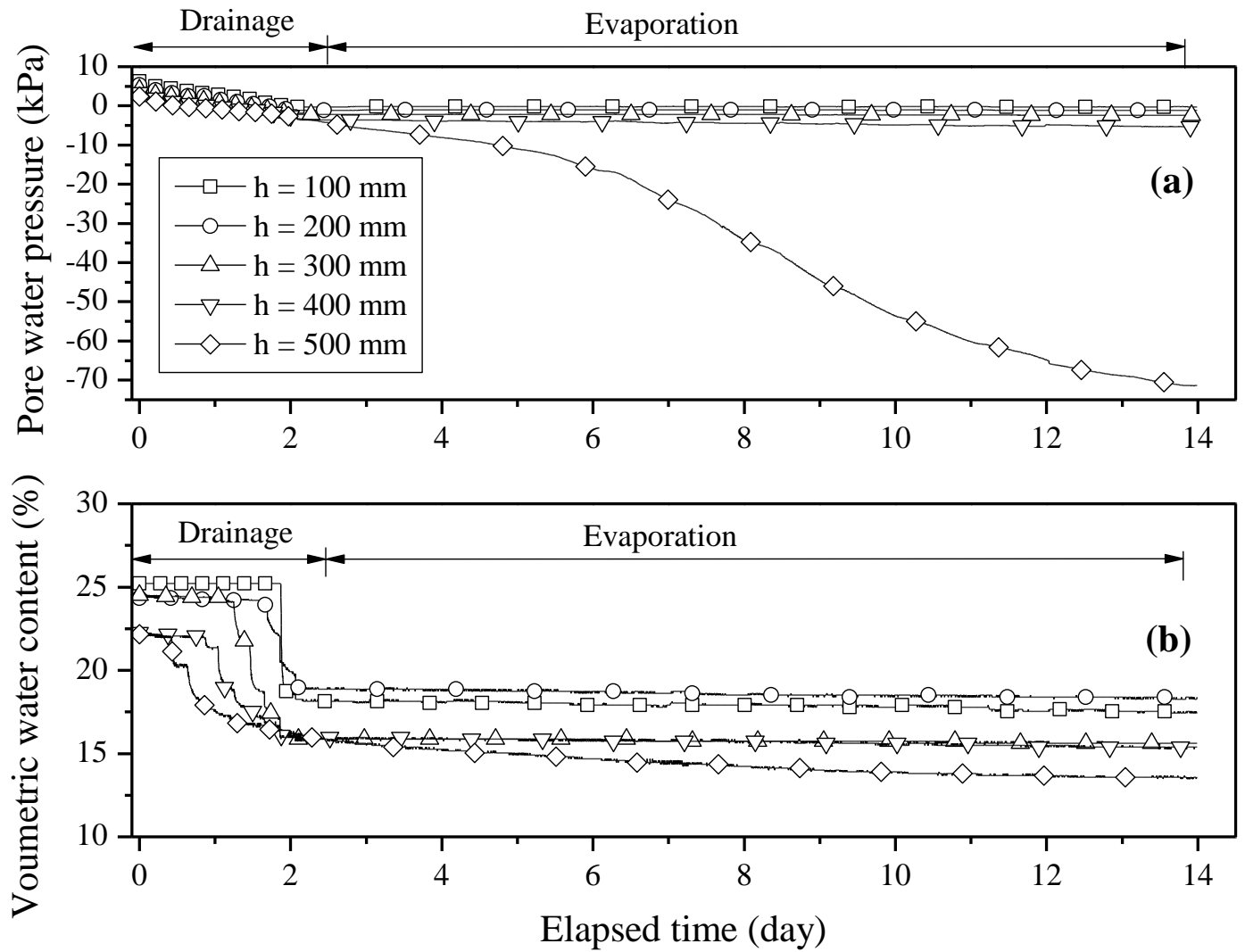
644 Fig. 7: Test on ITL_{10} : pore water pressure and volumetric water content evolutions during Drainage 1 and
 645 Evaporation 1

646



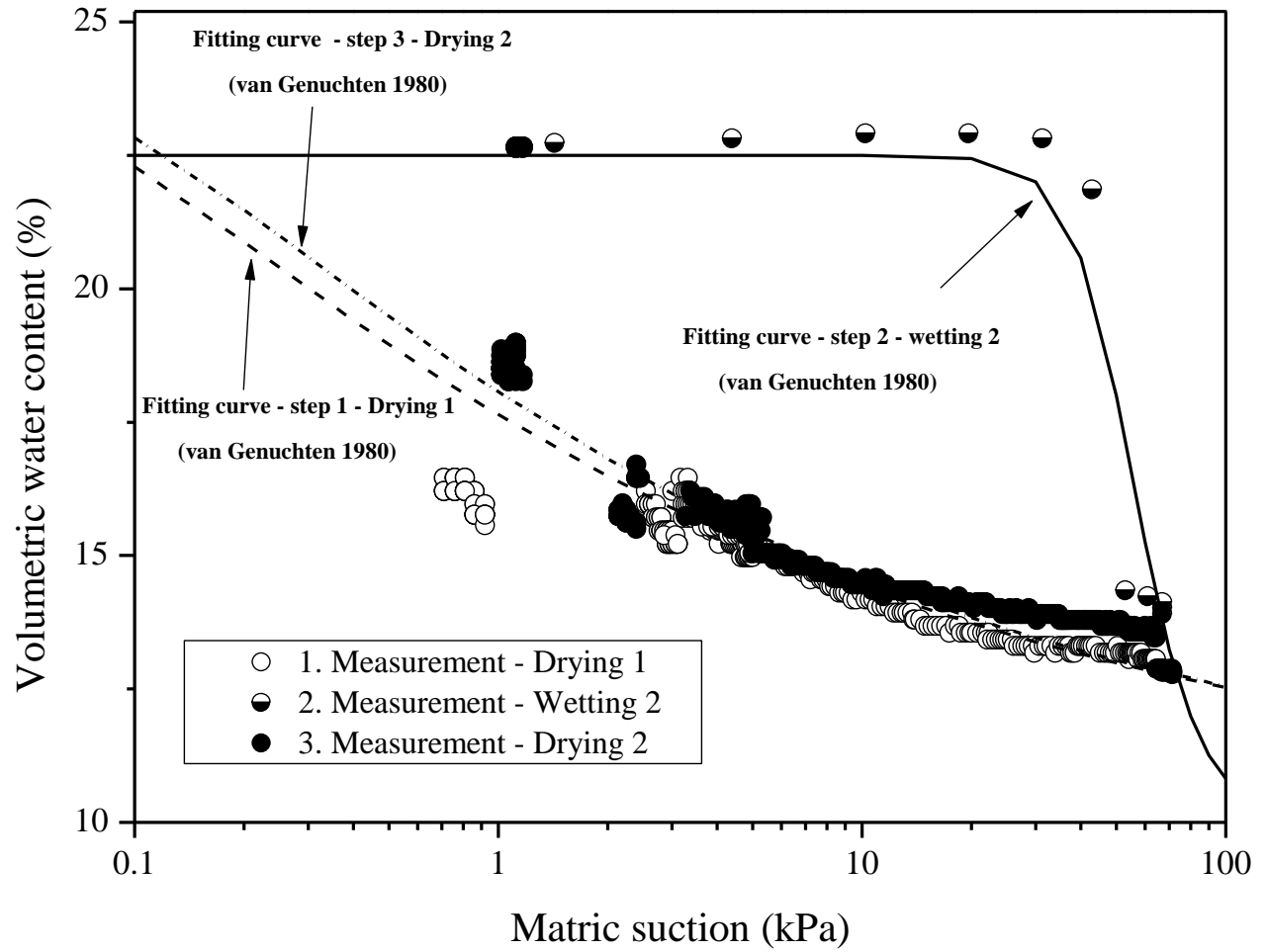
647

648 Fig. 8: Test on ITL_{10} : pore water pressure and volumetric water content evolutions in Saturation 2



649

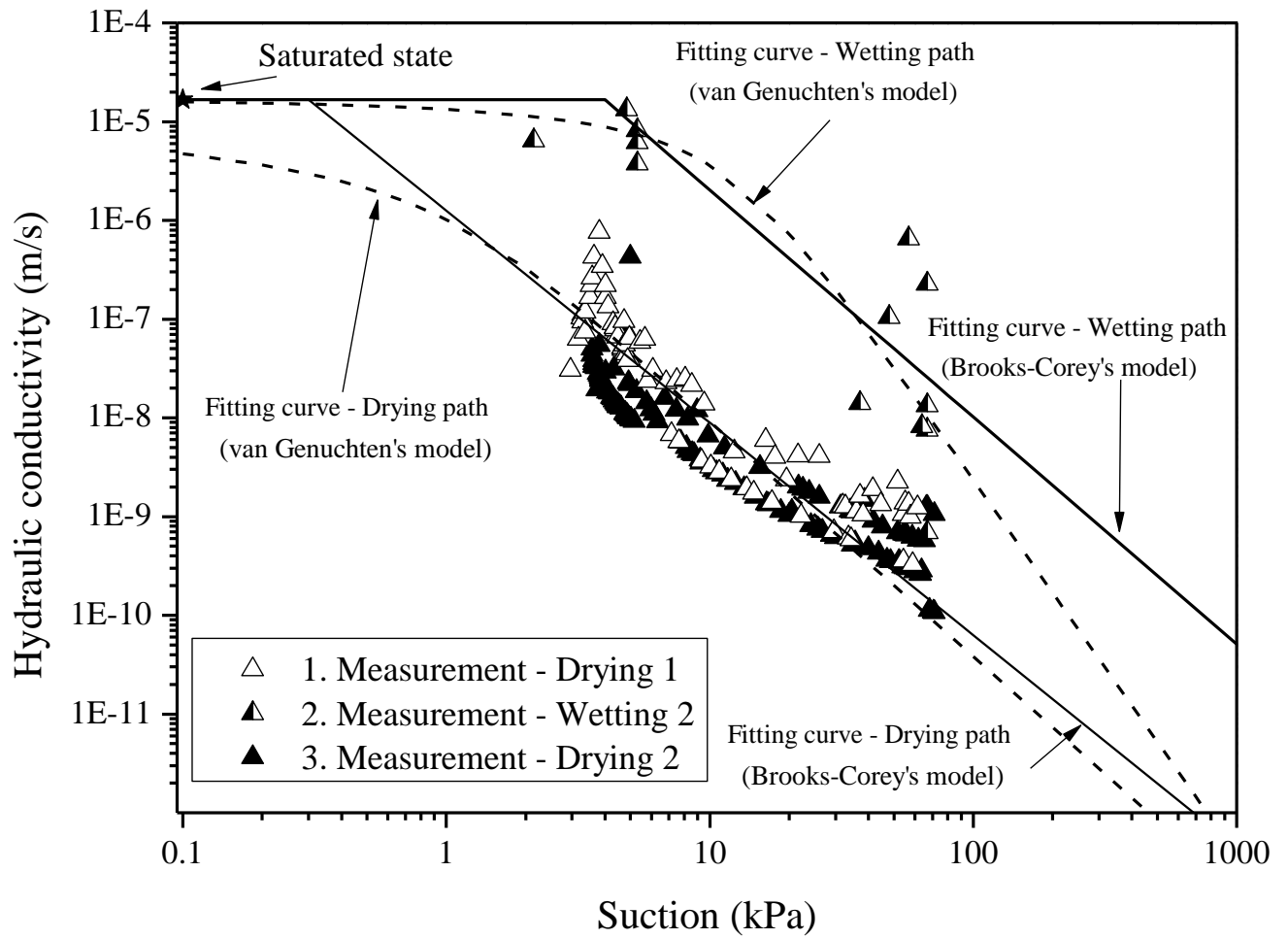
650 Fig. 9: Test on *ITL*₁₀: pore water pressure and volumetric water content evolutions during Drainage 2 and
 651 Evaporation 2



652

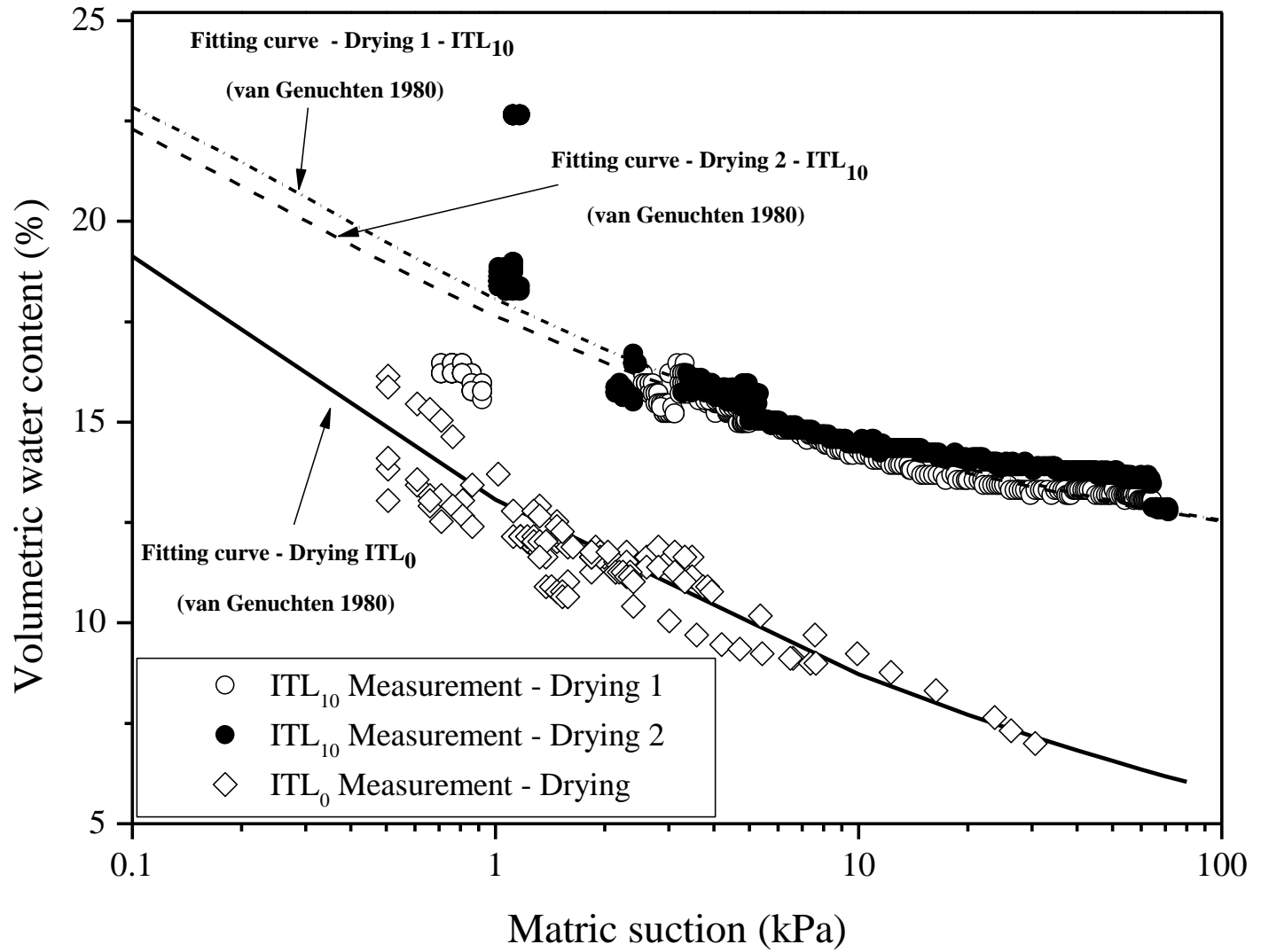
653 Fig. 10: WRC of ITL_{10} with fitting curves using the van Genuchten's model (1980)

654



655

656 Fig. 11: Hydraulic conductivity of ITL_{10} obtained with drying-wetting cycles

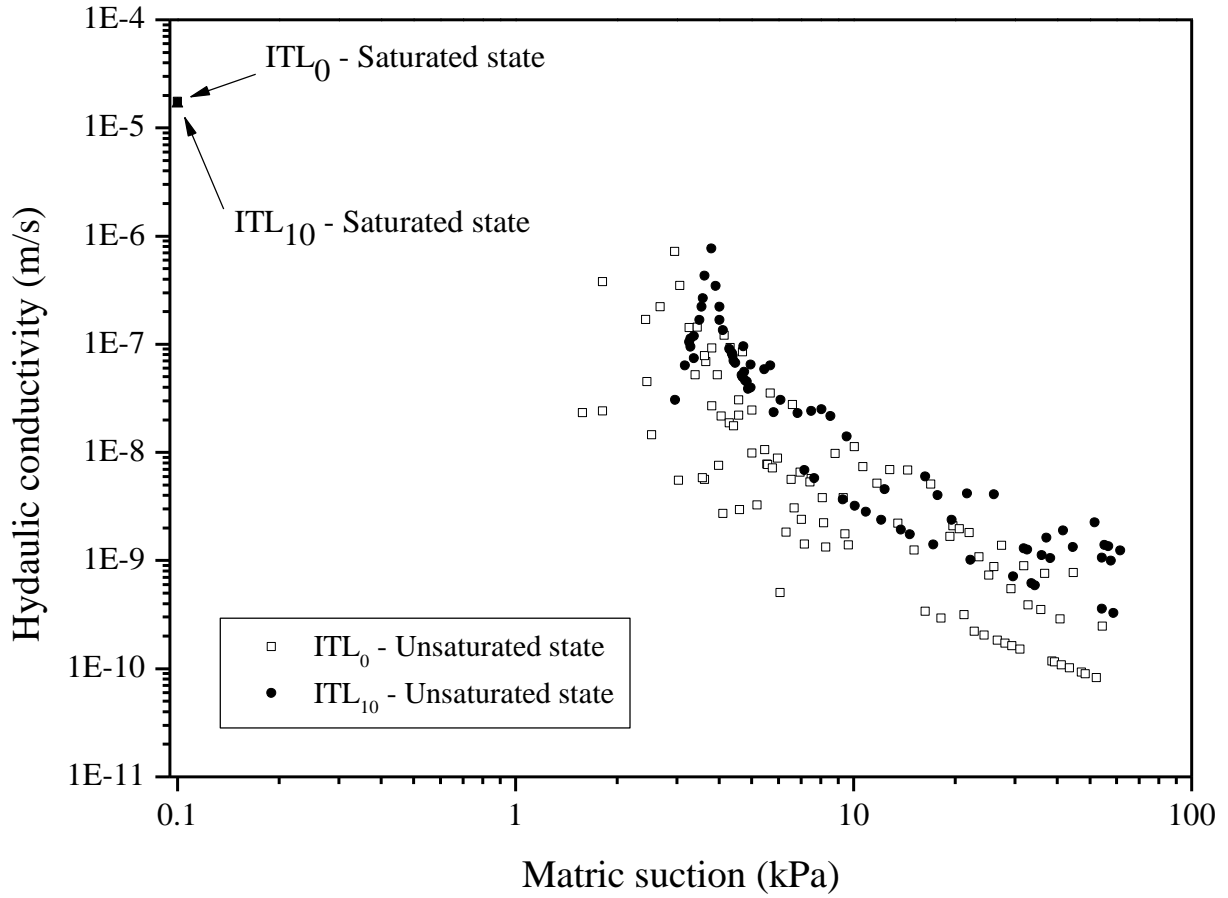


657

658 Fig. 12: Comparison of SWRC between ITL_0 and ITL_{10}

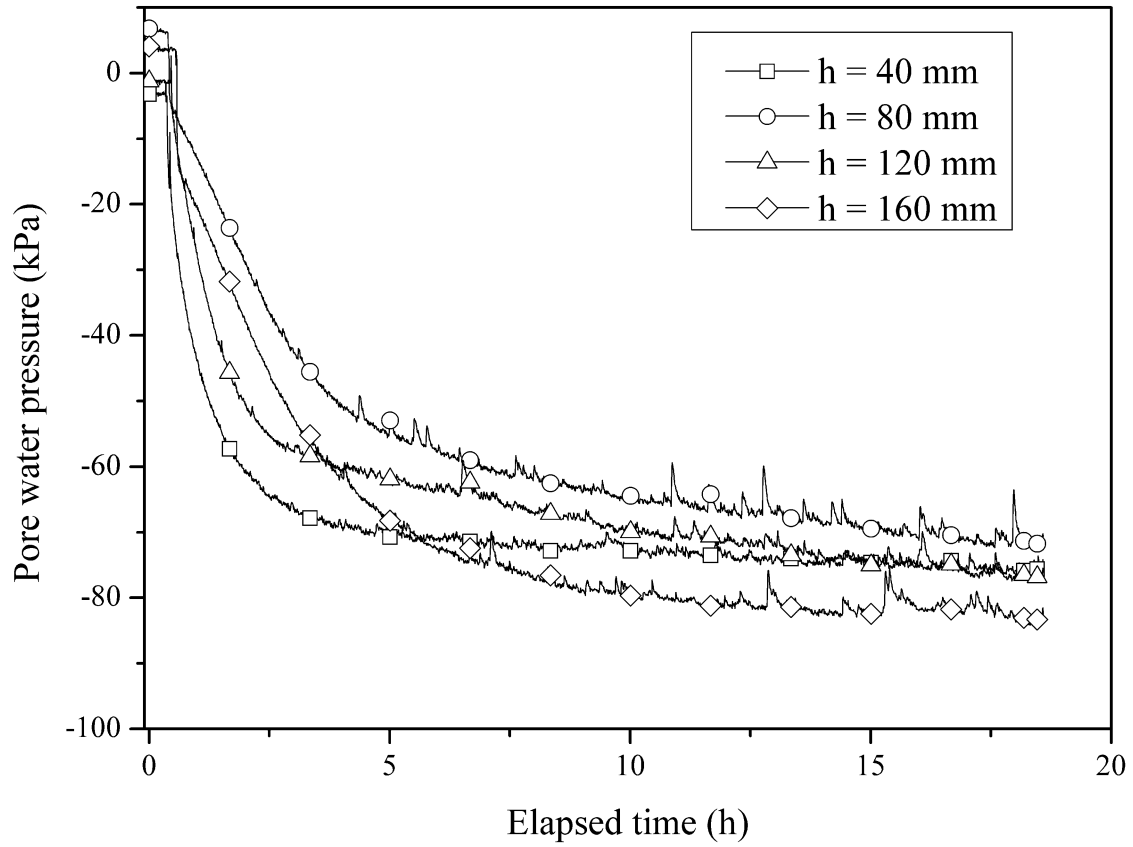
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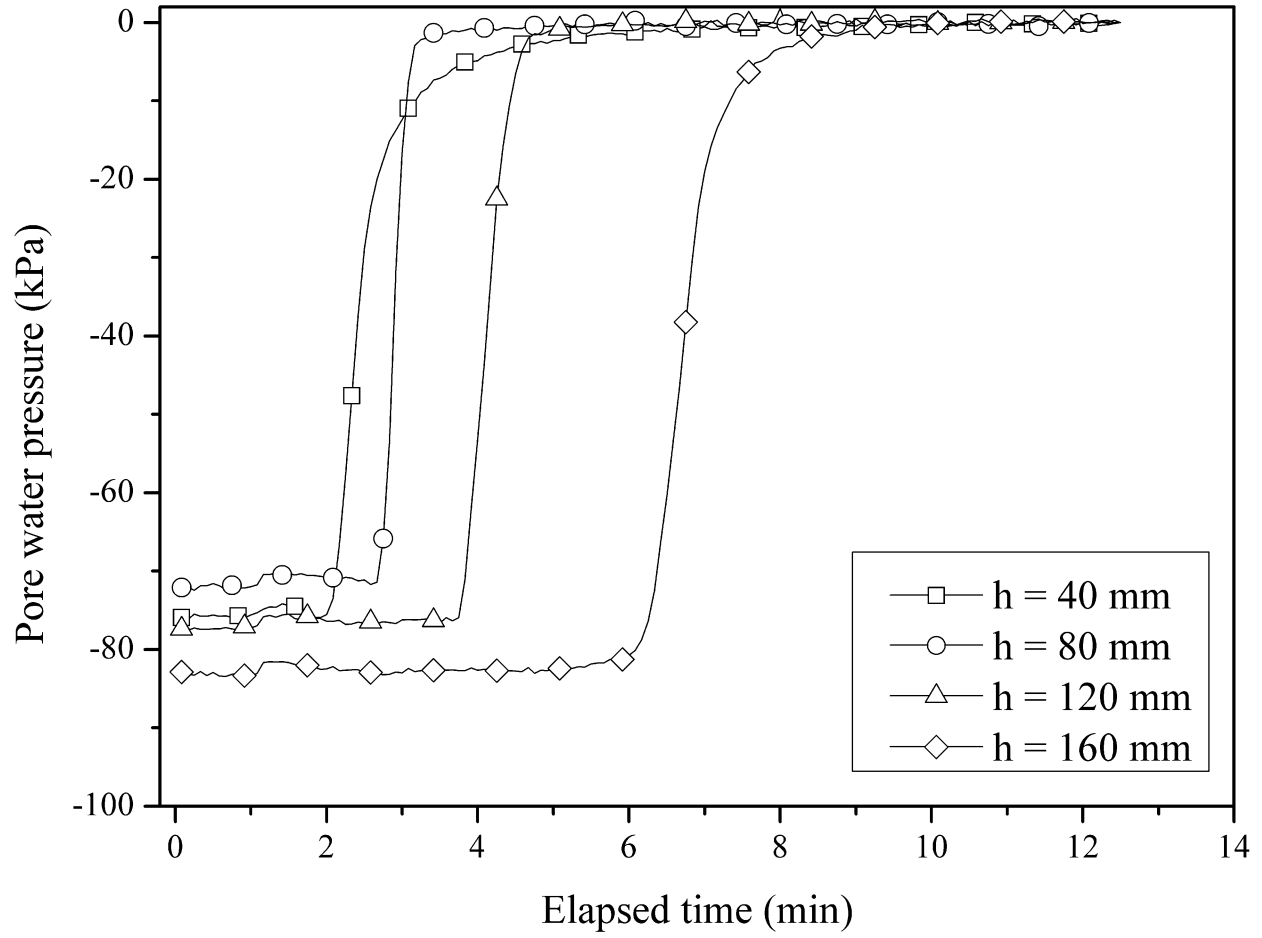
662 Fig. 13: Comparison of hydraulic conductivity between ITL_0 and ITL_{10}



663

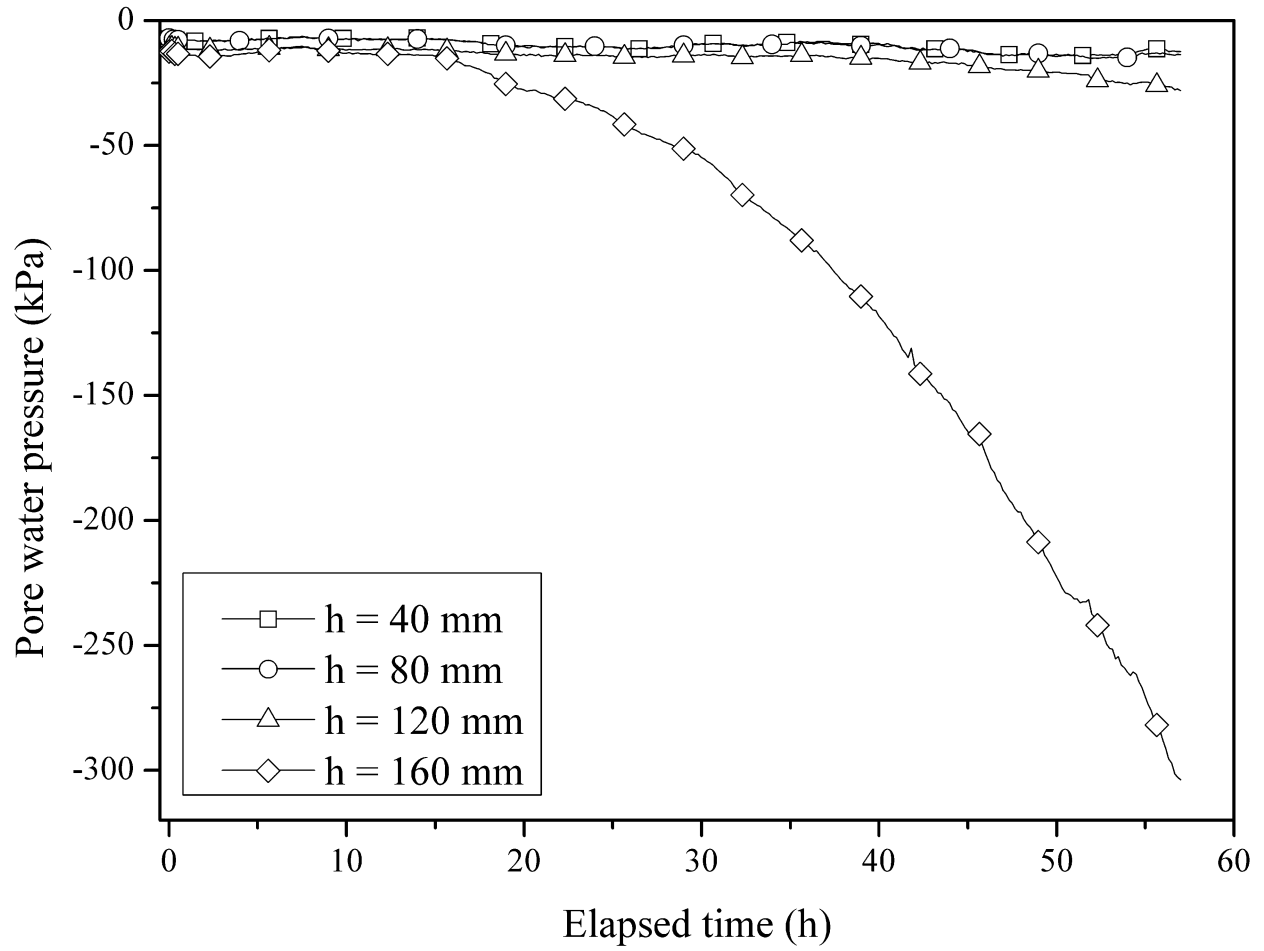
664 **Fig. 14: Test on *Fines*: suction stabilization after the installation of tensiometers**

665



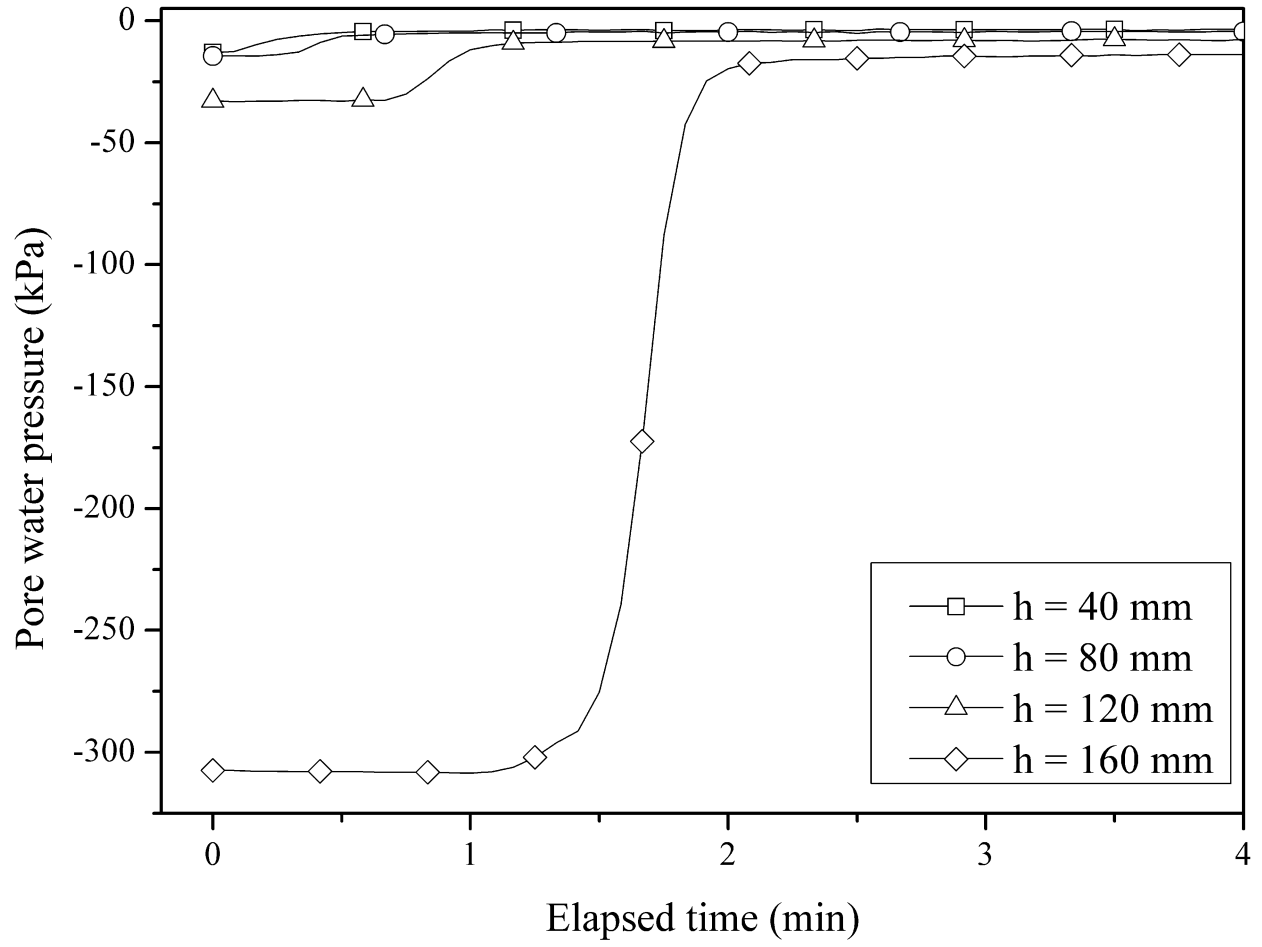
666

667 Fig. 15: Test on *Fines*: suction evolutions during Saturation 1



668

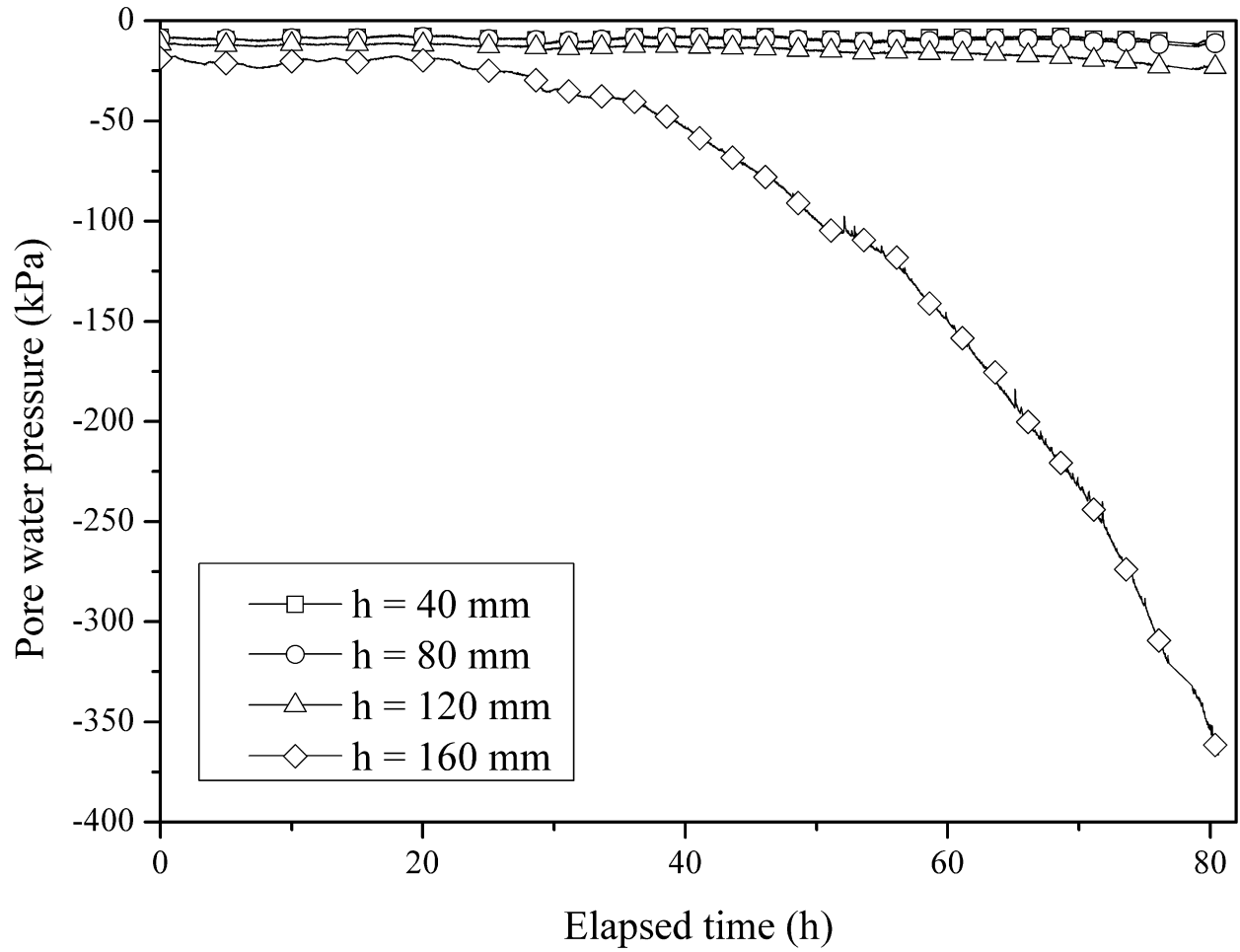
669 **Fig. 16: Test on *Fines*: suction evolutions during Evaporation 1**



670

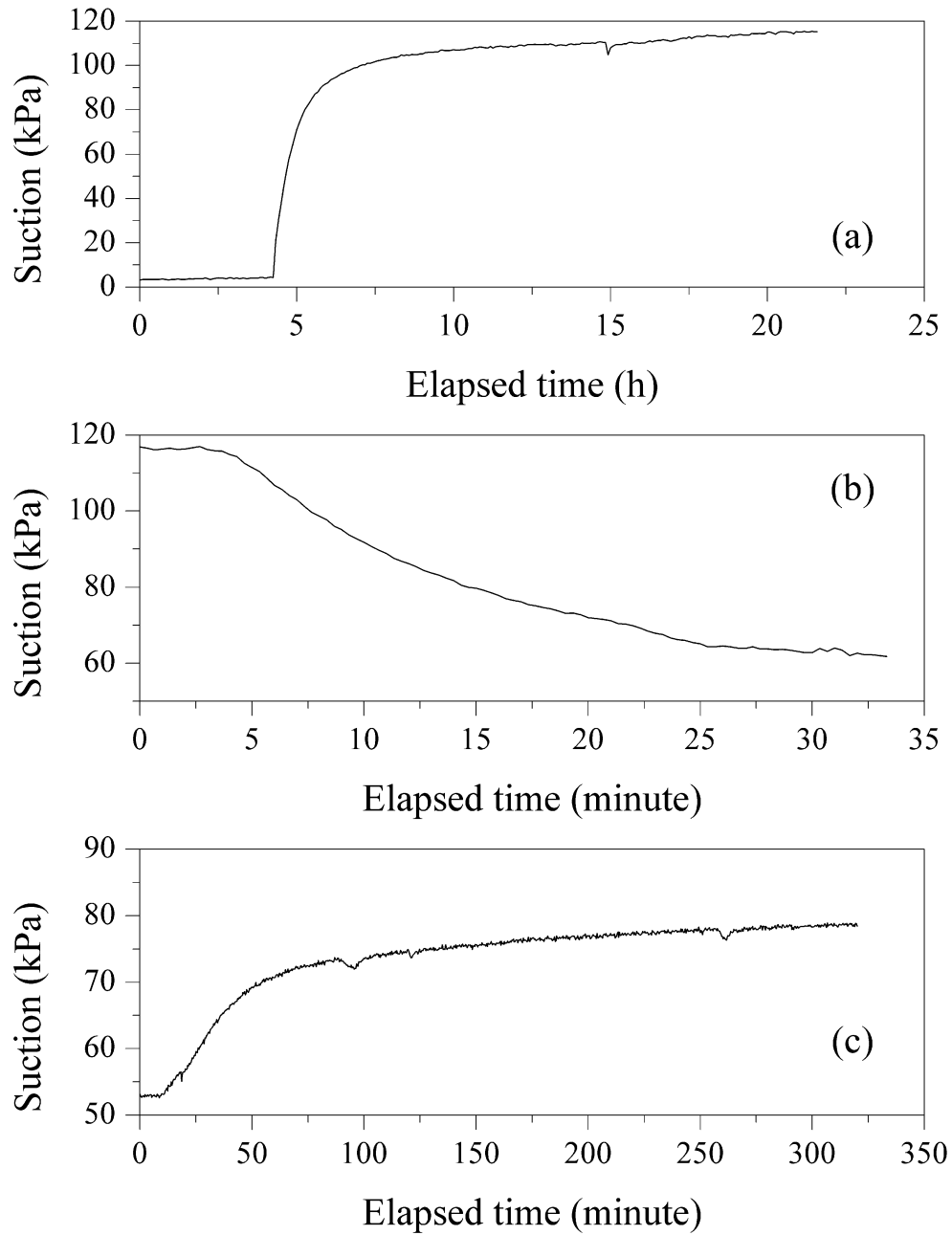
671 **Fig. 17: Test on *Fines*: suction evolutions during Saturation 2**

672



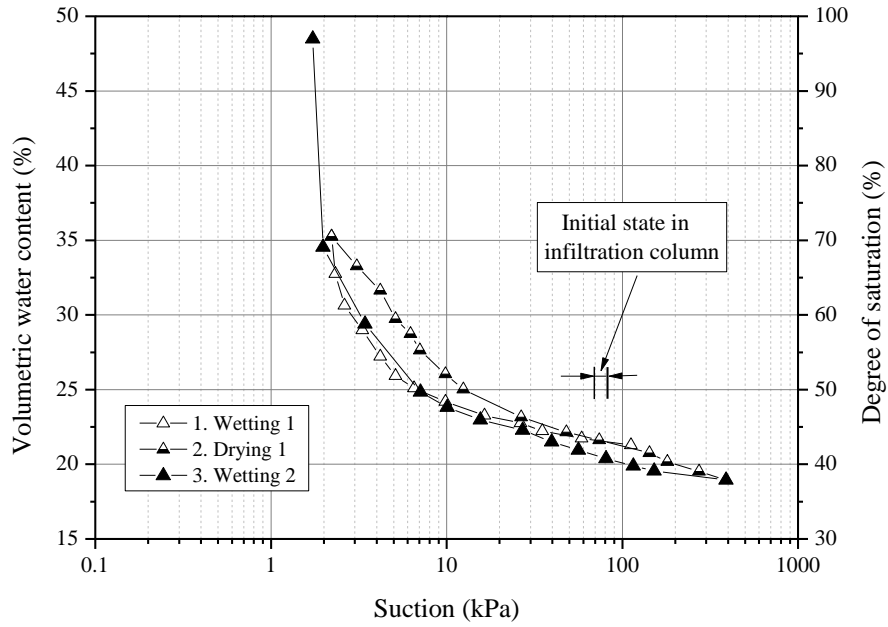
673

674 Fig. 18: Test on *Fines*: suction evolutions during Evaporation 2



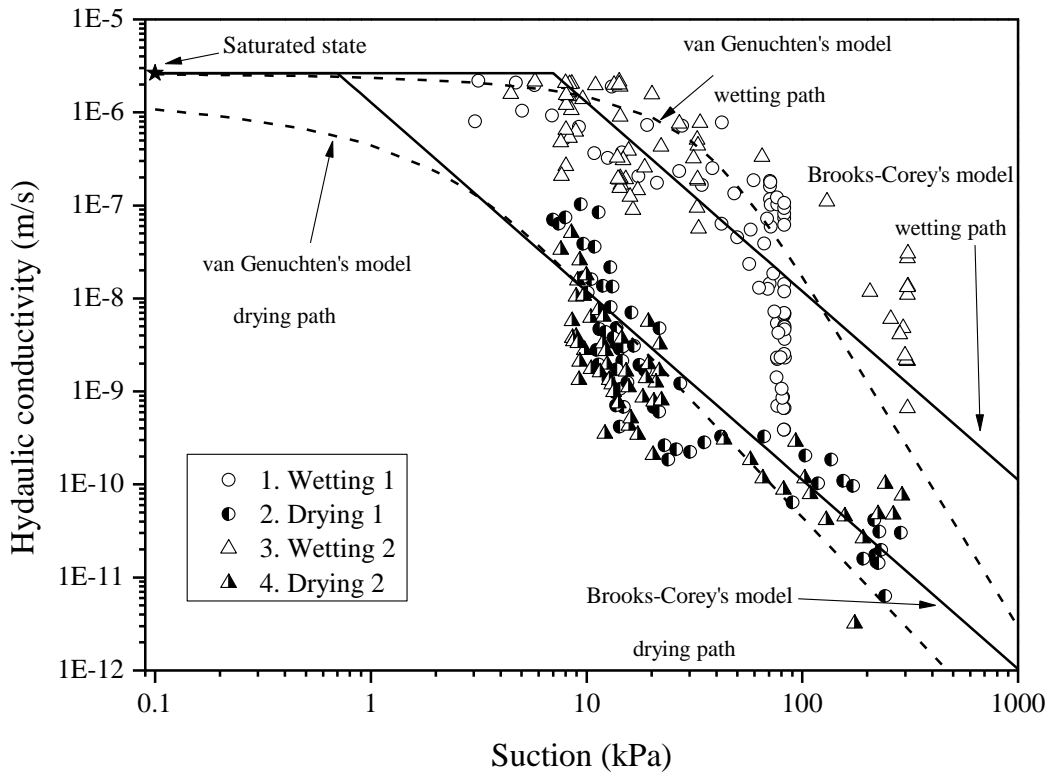
675

676 **Fig. 19: Stabilization of suction during the SWRC determination. (a) initial stabilization after tensiometer**
 677 **installation, (b) a wetting stage, (c) a drying stage**



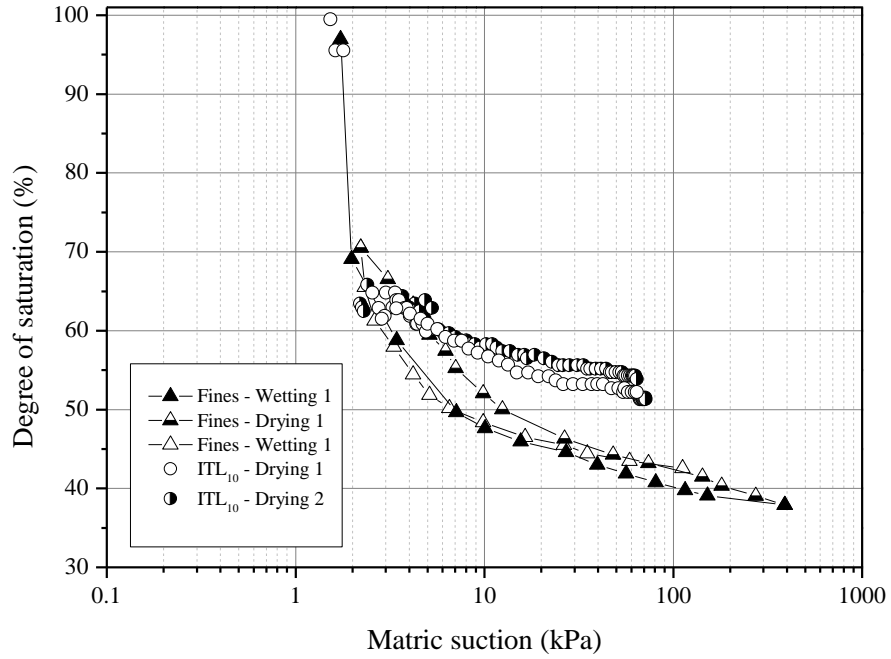
678

679 **Fig. 20: WRC of Fines**



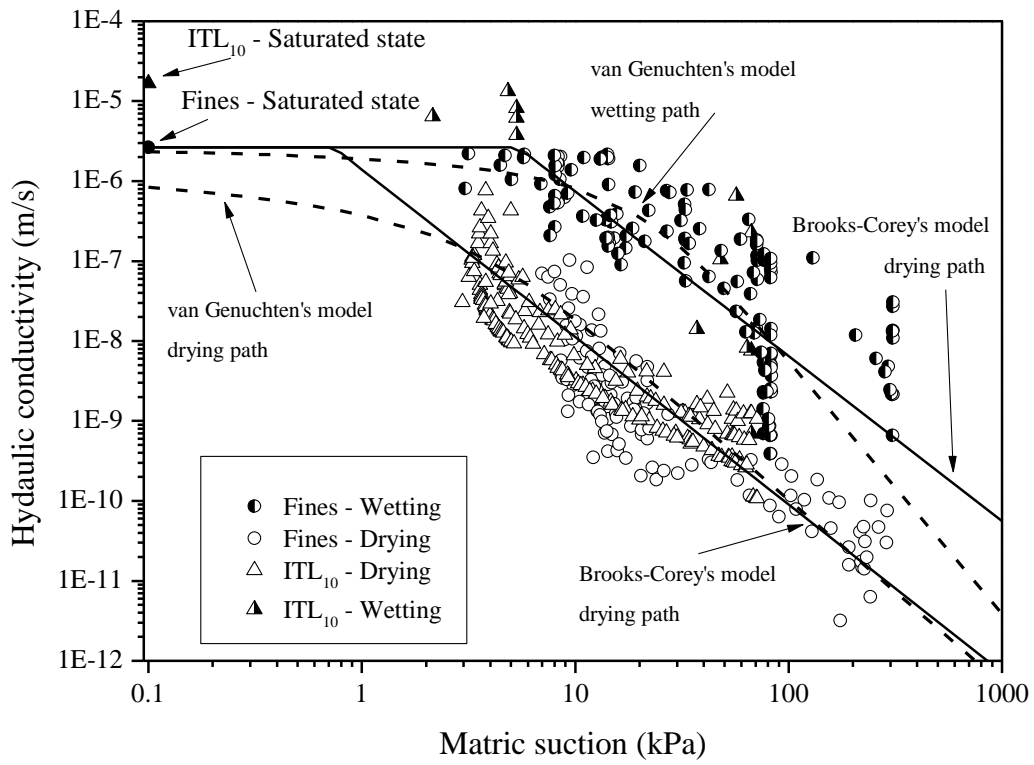
680

681 **Fig. 21: Hydraulic conductivity of Fines, obtained with drying/wetting cycles**



682

683 **Fig. 22: Comparison of SWRC between ITL_{10} and *Fines***



684

685 **Fig. 23: Comparison of hydraulic conductivity between ITL_{10} and *Fines***